Uni-axial compression; Other BC's

• Of particular interest (for use with stiffeners):



Uni-axial compression; 3 sides ss, one unloaded side free

 λ is chosen so that the "free" condition is represented (e.g. if λ=1=> simply-supported)



Uni-axial compression; 3 sides ss, one unloaded side free

• Current choice of w does not satisfy all BC's:

$$w(x = 0) = w(x = a) = 0$$
 OK

$$w(y = 0) = 0$$
 OK

$$M_{x} = -D_{11} \frac{\partial^{2} w}{\partial x^{2}} - D_{12} \frac{\partial^{2} w}{\partial y^{2}} = 0 \quad at \quad x = 0, a$$
 OK

$$M_{y} = -D_{12} \frac{\partial^{2} w}{\partial x^{2}} - D_{22} \frac{\partial^{2} w}{\partial y^{2}} = 0 \quad at \quad y = 0, b$$
 ???!!

Expression for the buckling load

$$N_{o} = \frac{\pi^{2} \Big[D_{11} m^{4} + 2(D_{12} + 2D_{66}) m^{2} \lambda^{2} (AR)^{2} + D_{22} (AR)^{4} \lambda^{4} \Big]}{a^{2} m^{2}}$$

Uni-axial compression; 3 sides ss, one unloaded side free

Comparison with exact solution



Infinitely long plate, 3 sides ss, one unloaded side free

exact solution:

 N_{xcrit}

$$=\frac{12D_{66}}{b^2}$$

approximate solution:







Buckling of rectangular plate under shear

simply supported plate^{*}



*Whitney, J.M., *Structural Analysis of Laminated Anisotropic Plates*, Technomic Publishing, 1987, section 5.7

Buckling under shear (cont'd)

- Galerkin solution
- Governing equation:

$$D_{11}\frac{\partial^4 w}{\partial x^4} + 2(D_{12} + 2D_{66})\frac{\partial^4 w}{\partial x^2 \partial y^2} + D_{22}\frac{\partial^4 w}{\partial y^4} = 2N_{xy}\frac{\partial^2 w}{\partial x \partial y}$$

• assume solution w in the form:

$$w = \sum \sum A_{mn} \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}$$
 characteristic functions

• multiply governing equation by the characteristic functions, integrate over the domain of the plate and set the result equal to zero (satisfied for m,n=1,2,...)

$$\iint \left[D_{11} \frac{\partial^4 w}{\partial x^4} + 2(D_{12} + 2D_{66}) \frac{\partial^4 w}{\partial x^2 \partial y^2} + D_{22} \frac{\partial^4 w}{\partial y^4} - 2N_{xy} \frac{\partial^2 w}{\partial x \partial y} \right] \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b} dx dy = 0$$

Buckling under shear (cont'd)

• Substituting for w and carrying out the integrations:

 $\pi^{4} \Big[D_{11}m^{4} + 2(D_{12} + 2D_{66})m^{2}n^{2}(AR)^{2} + D_{22}n^{4}(AR)^{4} \Big] A_{mn} - 32mn(AR)^{3}b^{2}N_{xy} \sum \sum T_{ij}A_{ij} = 0$ $T_{ij} = \frac{ij}{(m^{2} - i^{2})(n^{2} - j^{2})} \quad for \ m \pm i \ odd \ and \ n \pm j \ odd$ (AR)=a/b, aspect ratio $T_{ij} = 0 \quad otherwise$

- Equation breaks down to two sets of homogeneous equations, one for m+n odd and one for m+n even; each is of the form: [E]{A_{mn}}= N_{xy} [H] {A_{mn}}
- => two generalized eigenvalue problems
- Lowest eigenvalue corresponds to buckling load; note that for specially orthotropic plates the eigenvalues are in pairs of positive and negative numbers of same magnitude

Buckling under combined loads

 Combined compression and shear, simplysupported all around



Buckling under compression and shear

• approximate solution (2 terms):

$$w = w_1 \sin \frac{\pi x}{a} \sin \frac{\pi y}{b} + w_2 \sin \frac{2\pi x}{a} \sin \frac{2\pi y}{b}$$

Single term won't work for shear!

satisfies displacement (and force) BC's:

$$w(x = 0) = w(x = a) = 0$$

 $w(y = 0) = w(y = b) = 0$

$$M = -D \frac{\partial^2 w}{\partial w} = D \frac{\partial^2 w}{\partial w} = 0 \text{ at } \mathbf{x}$$

$$M_{x} = -D_{11} \frac{\partial x^{2}}{\partial x^{2}} - D_{12} \frac{\partial y^{2}}{\partial y^{2}} = 0 \text{ at } x = 0, a$$
$$M_{y} = -D_{12} \frac{\partial^{2} w}{\partial x^{2}} - D_{22} \frac{\partial^{2} w}{\partial y^{2}} = 0 \text{ at } y = 0, b$$

no need to satisfy force BC's for energy minimization approach (but need more terms)

• w_1, w_2 are unknown

Energy minimization

• Minimize:

$$\Pi_{c} = \frac{1}{2} \iint \left\{ D_{11} \left(\frac{\partial^{2} w}{\partial x^{2}} \right)^{2} + 2D_{12} \frac{\partial^{2} w}{\partial x^{2}} \frac{\partial^{2} w}{\partial y^{2}} + D_{22} \left(\frac{\partial^{2} w}{\partial y^{2}} \right)^{2} + 4D_{66} \left(\frac{\partial^{2} w}{\partial x \partial y} \right)^{2} + 4D_{16} \frac{\partial^{2} w}{\partial x^{2}} \frac{\partial^{2} w}{\partial x \partial y} + 4D_{26} \frac{\partial^{2} w}{\partial y^{2}} \frac{\partial^{2} w}{\partial x \partial y} \right\} dxdy + \frac{1}{2} \iint N_{x} \left(\frac{\partial w}{\partial x} \right)^{2} dxdy + \iint N_{xy} \frac{\partial w}{\partial x} \frac{\partial w}{\partial y} dxdy$$

- integrations carried over the entire plate
- Nx, Nxy constant

•
$$\frac{N_{xy}}{N_x} = k$$

•
$$D_{16} = D_{26} = 0$$

Some intermediate results

$$\left(\frac{\partial^2 w}{\partial x^2}\right)^2 = w_1^2 \frac{\pi^4}{4b^4} \left(1 - \cos\frac{2\pi x}{a}\right) \left(1 - \cos\frac{2\pi y}{b}\right) + w_2^2 \frac{16\pi^4}{4b^4} \left(1 - \cos\frac{4\pi x}{a}\right) \left(1 - \cos\frac{4\pi y}{b}\right) + 2w_1 w_2 \frac{4\pi^4}{b^4} \frac{1}{4} \left(\cos\frac{\pi x}{a} - \cos\frac{3\pi x}{a}\right) \left(\cos\frac{\pi y}{b} - \cos\frac{3\pi y}{b}\right)$$

- ... similarly for other integrands
- carrying out the integrations:

$$\int_{0}^{a} \int_{0}^{b} \left(\frac{\partial^{2} w}{\partial x^{2}}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{4}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{4}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial^{2} w}{\partial x^{2}}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{4}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{4}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial^{2} w}{\partial x^{2}}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{2}b^{2}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{2}b^{2}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial^{2} w}{\partial x}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{2}b^{2}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{2}b^{2}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial^{2} w}{\partial x}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{2}b^{2}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{2}b^{2}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial w}{\partial x}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{2}b^{2}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{2}b^{2}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial w}{\partial x}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{2}b^{2}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{2}b^{2}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial w}{\partial x}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{2}b^{2}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{2}b^{2}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial w}{\partial x}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{2}b^{2}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{2}b^{2}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial w}{\partial x}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{2}b^{2}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{2}b^{2}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial w}{\partial x}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{2}b^{2}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{2}b^{2}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial w}{\partial x}\right)^{2} dx dy = w_{1}^{2} \frac{\pi^{4}}{4a^{2}b^{2}} ab + w_{2}^{2} \frac{4\pi^{4}}{a^{2}b^{2}} ab \\ \int_{0}^{a} \int_{0}^{b} \left(\frac{\partial w}{\partial x}\right)^{2} dx dy = \frac{w_{1}w_{2}\pi^{2}}{2ab} \left(\frac{2a}{3\pi} + \frac{2a}{\pi}\right) \left(\frac{2b}{3\pi} - \frac{2b}{\pi}\right) + \frac{w_{1}w_{2}\pi^{2}}{2ab} \left(\frac{2a}{3\pi} - \frac{2a}{\pi}\right) \left(\frac{2b}{3\pi} + \frac{2b}{\pi}\right)$$

 $N_o = N_{xcrit} = -N_x$

• to determine w_1 , w_2 ,

$$\frac{\partial \Pi_c}{\partial w_1} = 0$$
$$\frac{\partial \Pi_c}{\partial w_2} = 0$$

System of equations

• which lead to

 $\frac{1}{2} \left\{ D_{11} \frac{w_1 \pi^4 b}{2a^3} + 2(D_{12} + 2D_{66}) \frac{\pi^4 w_1}{2ab} + D_{22} \frac{w_1 \pi^4 a}{2b^3} \right\} - N_o \frac{w_1 \pi^2 b}{4a} + \frac{32}{9} k N_o w_2 = 0$ $\frac{1}{2} \left\{ D_{11} \frac{8w_2 \pi^4 b}{a^3} + 2(D_{12} + 2D_{66}) \frac{8\pi^4 w_2}{ab} + D_{22} \frac{8w_2 \pi^4 a}{b^3} \right\} - N_o \frac{w_2 \pi^2 b}{a} + \frac{32}{9} k N_o w_1 = 0$

- <u>homogeneous</u> system of two eqns in the two unknowns w₁, w₂
- trivial solution w₁=w₂=0 corresponds to inplane deformations of the plate

System of equations

Setting

$$K_{1} = \frac{1}{4} \left[D_{11} \frac{\pi^{4}b}{a^{3}} + 2(D_{12} + 2D_{66}) \frac{\pi^{4}}{ab} + D_{22} \frac{\pi^{4}a}{b^{3}} \right]$$

• and using matrix notation eigenvalue $\begin{bmatrix} K_1 & 0 \\ 0 & 16K_1 \end{bmatrix} \begin{bmatrix} w_1 \\ w_2 \end{bmatrix} = \begin{bmatrix} \frac{\pi^2 b}{4a} & -\frac{32}{9}k \\ -\frac{32}{9}k & \frac{\pi^2 b}{a} \end{bmatrix} \begin{bmatrix} w_1 \\ w_2 \end{bmatrix}$

generalized eigen-value problem of the form:

$$A x = \alpha B x$$

Solution to the eigenvalue problem

pre-multiply both sides of the equation by <u>B</u>-1

$$\underbrace{B^{-1} A x}_{\sim} = \alpha B^{-1} B x$$

• obtain standard eigen value problem:

$$C_{\tilde{x}} = \alpha_{\tilde{x}}$$

• the eigenvalues are obtained as solutions to:

$$\det \left[\underbrace{C}_{\tilde{a}} - \alpha I_{\tilde{a}} \right] = 0$$

For our specific example

$$B^{-1} = \frac{1}{\frac{\pi^4 b^2}{4a^2} - \left(\frac{32}{9}k\right)^2} \left[\frac{\pi^2 b}{a} + \frac{32}{9}k \\ \frac{32}{9}k + \frac{\pi^2 b}{4a}\right]$$

and after some rearranging, the standardized eigenvalue problem has the form,



Determination of buckling load(s)

• the eigenvalue is the solution to:

$$\left(\frac{\pi^2 b}{a} - \alpha\right) \left(\frac{4\pi^2 b}{a} - \alpha\right) - \frac{512(32)}{81}k^2 = 0$$

• solving for α and substituting in terms of N_o:

$$N_{o} = \frac{\pi^{2}}{a^{2}} \frac{\left(D_{11} + 2(D_{12} + 2D_{66})\frac{a^{2}}{b^{2}} + D_{22}\frac{a^{4}}{b^{4}}\right)}{2 - \frac{8192}{81}\frac{a^{2}}{b^{2}\pi^{4}}k^{2}} \left[5 \pm \sqrt{9 + \frac{65536}{81}\frac{a^{2}}{\pi^{4}b^{2}}k^{2}}\right]$$

two solutions: use the lowest

Special cases: Pure compression

• Nxy=0 => k=0

$$N_o = \frac{\pi^2}{a^2} \left(D_{11} + 2(D_{12} + 2D_{66}) \frac{a^2}{b^2} + D_{22} \frac{a^4}{b^4} \right)$$

comparing with exact solution found earlier, this expression is identical to the exact solution for m=1 (but not so accurate for m>1)

Special case: Comparison to exact solution





Special cases: Pure Shear

• set k very large



Typically, 27% higher than exact solution!

Shear buckling: Comparison of various methods⁽¹⁾



(1) S. Simonian & C. de Winter

K relation: equation just derived

Shear buckling: Comparison of various methods



Shear buckling: Comparison of various methods



Shear buckling: Comparison of various methods



Interaction curve: Buckling under combined compression and shear

 Even though the present solution is approximate, it is expected to be quite accurate in providing the interaction curve when both compression and shear are applied

Buckling interaction curve: Combined compression and shear



Here, N_{xcrit} and N_{xycrit} refer to the buckling when each load is applied <u>individually</u>



Buckling under various loads and Boundary Conditions⁽¹⁾

$ \rightarrow \begin{array}{c} \uparrow & ss \\ ss & b \\ \downarrow & ss \end{array} $		$N_o = \frac{\pi^2 \left[D_{11} m^4 + 2(D_{12} + 2D_{66}) m^2 (AR)^2 + D_{22} (AR)^4 \right]}{a^2 m^2}$
$ \rightarrow \begin{array}{c} \uparrow & ss \\ c & b \\ \psi & ss \end{array} \begin{array}{c} c \\ \bullet \\ \end{array} $	$\lambda = \frac{a}{b} \left(\frac{D_{22}}{D_{11}} \right)^{1/4}$	$N_{o} = \frac{\pi^{2}}{b^{2}} \sqrt{D_{11}D_{22}} (K)$ $K = \frac{4}{\lambda^{2}} + \frac{2(D_{12} + 2D_{66})}{\sqrt{D_{11}D_{22}}} + \frac{3}{4}\lambda^{2} \qquad 0 < \lambda < 1.662$ $K = \frac{m^{4} + 8m^{2} + 1}{\lambda^{2}(m^{2} + 1)} + \frac{2(D_{12} + 2D_{66})}{\sqrt{D_{11}D_{22}}} + \frac{\lambda^{2}}{m^{2} + 1} \qquad \lambda > 1.662$
$\rightarrow \begin{array}{c} ss & \uparrow & c \\ b & b \\ \psi & c \end{array} \qquad \qquad$	$\lambda = \frac{a}{b} \left(\frac{D_{22}}{D_{11}} \right)^{1/4}$	$N_{o} = \frac{\pi^{2}}{b^{2}} \sqrt{D_{11}D_{22}}(K) \qquad K = \frac{m^{2}}{\lambda^{2}} + \frac{2(D_{12} + 2D_{66})}{\sqrt{D_{11}D_{22}}} + \frac{16}{3} \frac{\lambda^{2}}{m^{2}}$
$ \rightarrow \begin{array}{c} \uparrow & c \\ c & b \\ \psi & c \end{array} \leftarrow $	$\lambda = \frac{a}{b} \left(\frac{D_{22}}{D_{11}} \right)^{1/4}$	$N_{o} = \frac{\pi^{2}}{b^{2}} \sqrt{D_{11}D_{22}} (K) K = \frac{4}{\lambda^{2}} + \frac{8(D_{12} + 2D_{66})}{3\sqrt{D_{11}D_{22}}} + 4\lambda^{2} 0 < \lambda < 1.094$ $K = \frac{m^{4} + 8m^{2} + 1}{\lambda^{2}(m^{2} + 1)} + \frac{2(D_{12} + 2D_{66})}{\sqrt{D_{11}D_{22}}} + \frac{\lambda^{2}}{m^{2} + 1} \qquad \lambda > 1.094$
$ \xrightarrow{\text{ss}} b \qquad \text{ss} \\ \downarrow \qquad \text{ss} $	$\lambda = \frac{a}{b} \left(\frac{D_{22}}{D_{11}}\right)^{1/4}$	$N_{o} = \frac{\pi^{2}}{b^{2}} \sqrt{D_{11}D_{22}}(K) \qquad \qquad K = \frac{12}{\pi^{2}} \frac{D_{66}}{\sqrt{D_{11}D_{22}}} + \frac{1}{\lambda^{2}}$

(1) NASA/DoD Adv Composites Design Guide, vol II, 1983

Buckling under various loads and boundary conditions⁽¹⁾



(1) NASA/DoD Adv Composites Design Guide, vol II, 1983

Effect of BC's on buckling load of a square plate under compression



Biggest difference is less than 2.5 to 1 (compare to beams with 4 to 1 ratio)