

LINEAR MODELLING (INCL. FEM)  
AE4ASM003  
P1-2015

LECTURE 5  
29.09.2015

# TODAY...

- Isoparametric formulation
- Constant strain triangular elements
- Bending elements

# ISOPARAMETRIC FORMULATION

# FORMULATION PROPERTIES

- same interpolation functions for geometry and displacement
- tedious/confusing!
- simpler for computer program formulation
- extended application to all element types
- idealisation of elements with curved boundaries

# NATURAL OR GENERALISED COORDINATE SYSTEM

Let us assume a polynomial for the geometry

$$x = a_0 + a_1 r \quad \longrightarrow (1)$$

$$\text{at } x = x_1, \quad r = -1 \quad \longrightarrow (2)$$

$$\text{at } x = x_2, \quad r = +1$$

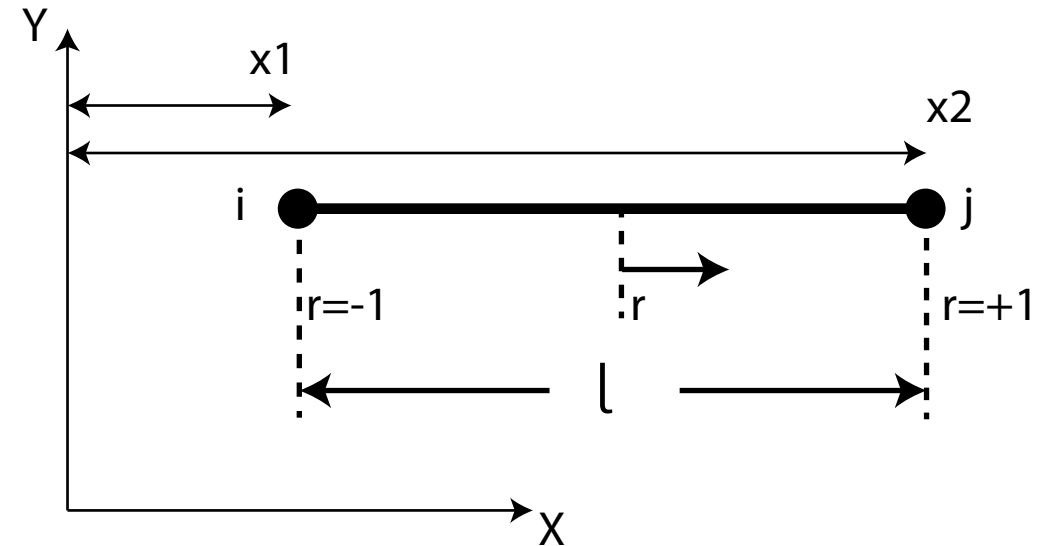
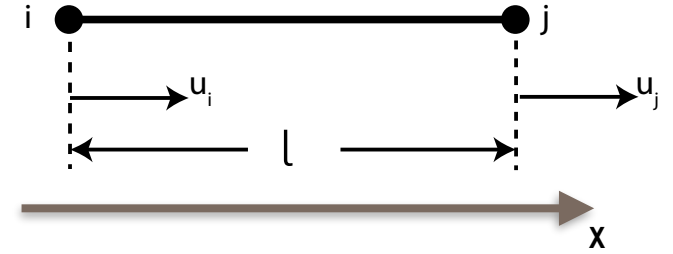
Solving and substituting back into x,

$$x = \frac{x_1 + x_2}{2} + \frac{x_2 - x_1}{2} \cdot r \quad \longrightarrow (3)$$

$$= \frac{1-r}{2} \cdot x_1 + \frac{1+r}{2} \cdot x_2 \quad \longrightarrow (4)$$

Therefore,

$$x = N_1 x_1 + N_2 x_2 \quad \text{where} \quad N_1 = \frac{1-r}{2}; \quad N_2 = \frac{1+r}{2}$$



$r$  is the natural coordinate system

This natural coordinate is attached to the bar no matter how it is oriented or placed in space

Therefore, a transformation to the global coordinate system must be made - Transformation mapping!

So, let's carry out the steps of the finite element analysis using this formulation:

Step 1: interpolation model

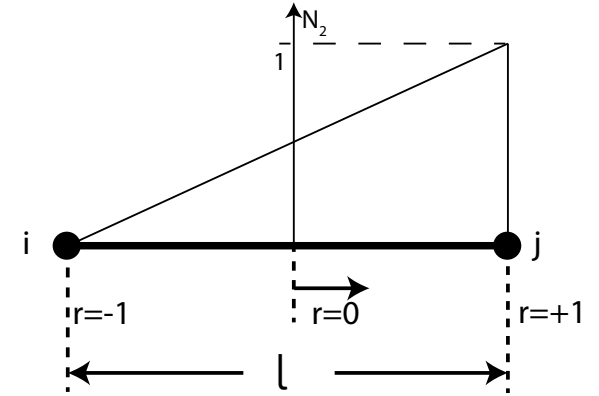
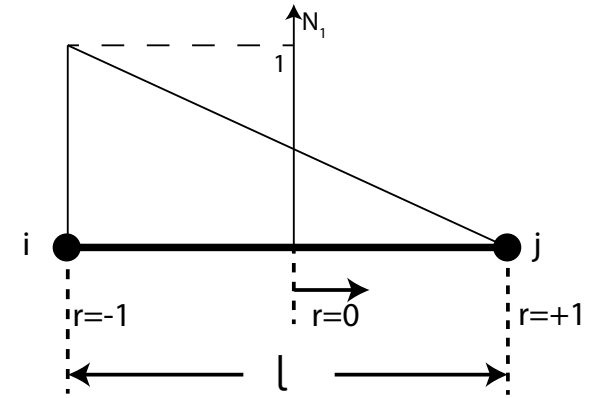
Step 2: strain and stress of the system

Step 3: stiffness matrix definition

Step 4: transformation

Step 5: load vector

Step 6: results



Step 2: strain and stress of the system

$$\begin{aligned}x &= N_1 x_1 + N_2 x_2 \\u &= N_1 u_1 + N_2 u_2\end{aligned} \longrightarrow (5)$$

Strain in local coordinate

$$\epsilon_r = \frac{du}{dr} = \frac{du}{dx} \cdot \frac{dx}{dr} \longrightarrow (6)$$

Strain in global coordinate

$$\epsilon_x = \frac{du}{dx} = \frac{du}{dr} / \frac{dx}{dr} \longrightarrow (7)$$

where  $\frac{du}{dr} = \frac{u_2 - u_1}{2}$  and  $\frac{dx}{dr} = \frac{x_2 - x_1}{2} = \frac{l}{2} \longrightarrow (8)$

Substituting in (7),

$$\epsilon_x = \left[ -\frac{1}{l} \quad \frac{1}{l} \right] \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} \longrightarrow (9)$$

Therefore, by definition,  $[B] = \begin{bmatrix} -\frac{1}{l} & \frac{1}{l} \end{bmatrix}$  and, stress  $\sigma = E [B]\{u\}$   $\longrightarrow$  (10)

In this case, the matrix B is simple because of the choice of the element.

In general, for higher order elements, B will be a function of the natural coordinate.

We will notice this difference for other elements such as higher order beams, triangular or quadrilateral elements.

### Step 3: stiffness matrix definition

We know that  $k = \int_0^l [B]^T E [B] A. dx$   $\longrightarrow$  (11)

### Step 4: transformation

From the reasoning above, B is generally a function of r, and therefore, the coordinate must be transformed for integration

$$\int_0^l f(x). dx = \int_{-1}^1 f(r). |[J]| dr \quad \text{where } |[J]| = J \quad \text{for a 1D case} \quad \longrightarrow \quad (12)$$

**Jacobian**



So, for the current assumed problem

$$J = \frac{dx}{dr} = \frac{l}{2} \longrightarrow (13)$$

Substituting it back into (11)

$$\begin{aligned} k &= \frac{l}{2} \int_{-1}^1 [B]^T E [B] A. dr \\ &= \frac{AE}{l} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \longrightarrow (14) \end{aligned}$$

\*\*\* For higher order elements, this integration is near-impossible so some numerical integration techniques are used

## Step 5: load vector

Body forces

$$\begin{aligned}\{f_b\} &= \int_V [N]^T \{X_b\} dV \\ &= \int_x [N]^T \{X_b\} A dx && \longrightarrow (15) \\ &= \frac{ALX_b}{2} \begin{Bmatrix} 1 \\ 1 \end{Bmatrix}\end{aligned}$$

Surface forces

$$\begin{aligned}\{f_s\} &= \int_S [N_s]^T \{T_x\} dS \\ &= \int_x [N_s]^T \{T_x\} dx && \longrightarrow (16) \\ &= \{T_x\} \frac{L}{2} \begin{Bmatrix} 1 \\ 1 \end{Bmatrix}\end{aligned}$$

# PLATE ANALYSIS

# TYPES OF PLATE ELEMENTS

- Any plate under in-plane and transverse loads will deform in 3 spatial coordinates
- Assuming the linear small deflection theory, uncoupling occurs
  - in-plane (membrane) forces
  - transverse (bending) forces

# MEMBRANE ELEMENTS - CONSTANT STRAIN TRIANGLES

The same process has to be followed:

Step 1: interpolation model

Step 2: strain and stress of the system

Step 3: stiffness matrix definition

Step 4: transformation

Step 5: load vector

Step 6: results

Step 1: interpolation model

$$u(x, y) = a_1 + a_2x + a_3y$$

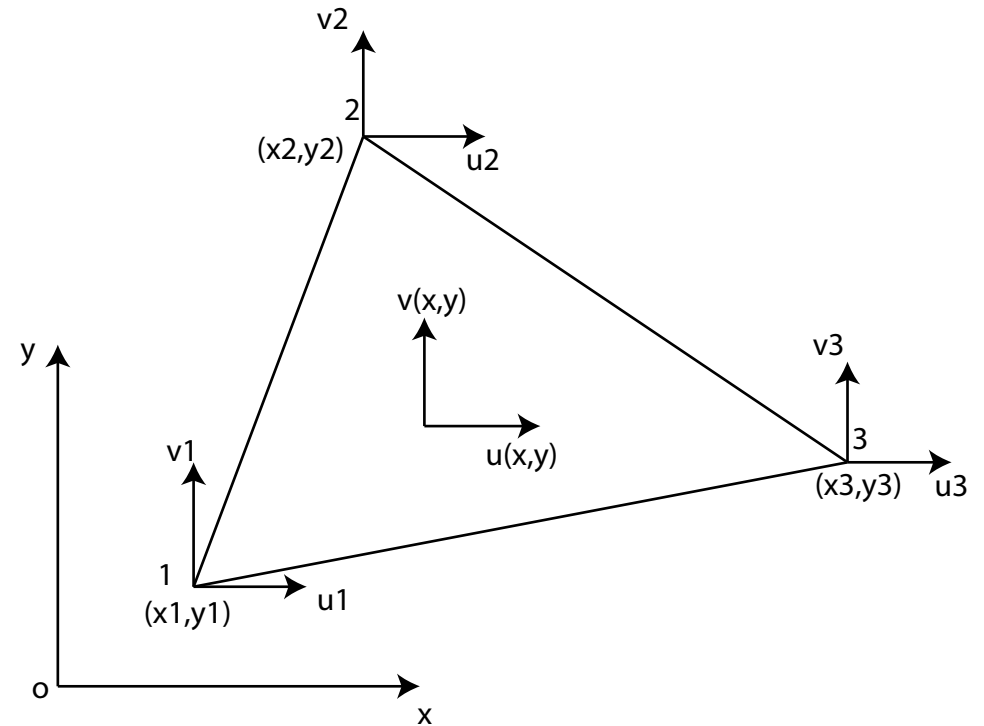
$$v(x, y) = a_4 + a_5x + a_6y$$

where

$$u(x, y) = u_1 \quad \& \quad v(x, y) = v_1 \quad \text{at } (x_1, y_1)$$

$$u(x, y) = u_2 \quad \& \quad v(x, y) = v_2 \quad \text{at } (x_2, y_2)$$

$$u(x, y) = u_3 \quad \& \quad v(x, y) = v_3 \quad \text{at } (x_3, y_3)$$



Solving for the constants and substituting and rearranging,

$$\vec{U} = \begin{Bmatrix} u(x, y) \\ v(x, y) \end{Bmatrix} = [N]\vec{u}^e \quad \longrightarrow \text{(iii)}$$

where

$$[N(x, y)] = \begin{bmatrix} N_1 & 0 & N_2 & 0 & N_3 & 0 \\ 0 & N_1 & 0 & N_2 & 0 & N_3 \end{bmatrix}$$

and,

$$N_1 = \frac{1}{2A} [y_{32}(x - x_2) - x_{32}(y - y_2)]$$

$$N_2 = \frac{1}{2A} [-y_{31}(x - x_3) + x_{31}(y - y_3)]$$

$$N_3 = \frac{1}{2A} [y_{21}(x - x_1) - x_{21}(y - y_1)]$$

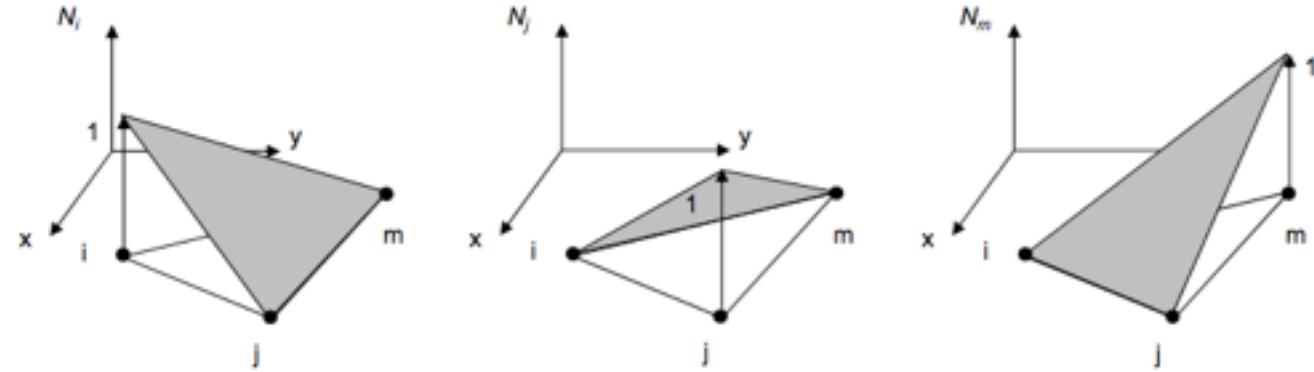
$$A = \frac{1}{2} (x_{32}y_{21} - x_{21}y_{32}) = \text{area of triangle 123}$$

$$x_{ij} = x_i - x_j$$

$$y_{ij} = y_i - y_j$$

$$\vec{u}^e = \begin{Bmatrix} u_1 \\ v_1 \\ u_2 \\ v_2 \\ u_3 \\ v_3 \end{Bmatrix}^e \quad \longrightarrow \text{(iv)}$$

Some requirements for the displacement model



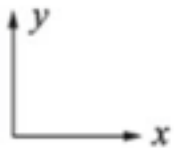
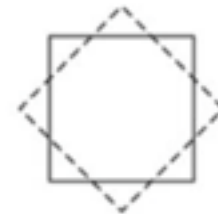
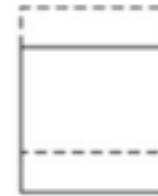
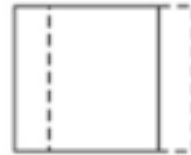
$u, v$  should yield constant value for a rigid body displacement

Therefore,

$$N_1 + N_2 + N_3 = 1$$



(v)



for all  $x, y$  locations



From solid mechanics,

$$\vec{\epsilon} = \begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{xy} \end{Bmatrix} = \begin{Bmatrix} \partial u / \partial x \\ \partial u / \partial y \\ \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \end{Bmatrix} \longrightarrow \text{(vi)}$$

which can be written in terms of nodal displacements as,

$$\vec{\epsilon} = [B] \vec{u}^e \longrightarrow \text{(vii)}$$

where

$$[B] = \frac{1}{2A} \begin{bmatrix} y_{32} & 0 & -y_{31} & 0 & y_{21} & 0 \\ 0 & -x_{32} & 0 & x_{31} & 0 & -x_{21} \\ -x_{32} & y_{32} & x_{31} & -y_{31} & -x_{21} & y_{21} \end{bmatrix}$$

For a plane stress condition,

$$\vec{\sigma} = [D] \vec{\epsilon}$$

where

$$\vec{\sigma} = \begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} \quad \text{and,} \quad [D] = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \longrightarrow \text{(viii)}$$

Stiffness matrix

$$[k^e] = \iiint_V [B]^T [D] [B] dV$$

Since in this case, the matrices B and D are constants and are independent of spatial coordinates, the thickness can be extracted to give,

$$[k^e] = [B]^T [D] [B] t \iint_A dA = tA [B]^T [D] [B] \longrightarrow \text{(ix)}$$

Transformation?

In-plane

$$[\lambda] = \begin{bmatrix} \underline{\lambda} & \underline{0} & \underline{0} \\ \underline{0} & \underline{\lambda} & \underline{0} \\ \underline{0} & \underline{0} & \underline{\lambda} \end{bmatrix}$$

Stiffness matrix, load vector, displacements!

$\longrightarrow$  (x)

where  $\underline{\lambda} = \begin{bmatrix} \cos\theta & \sin\theta \\ -\sin\theta & \cos\theta \end{bmatrix}$  and,  $\underline{0} = \begin{bmatrix} 0 & 0 \\ 0 & 0 \end{bmatrix}$

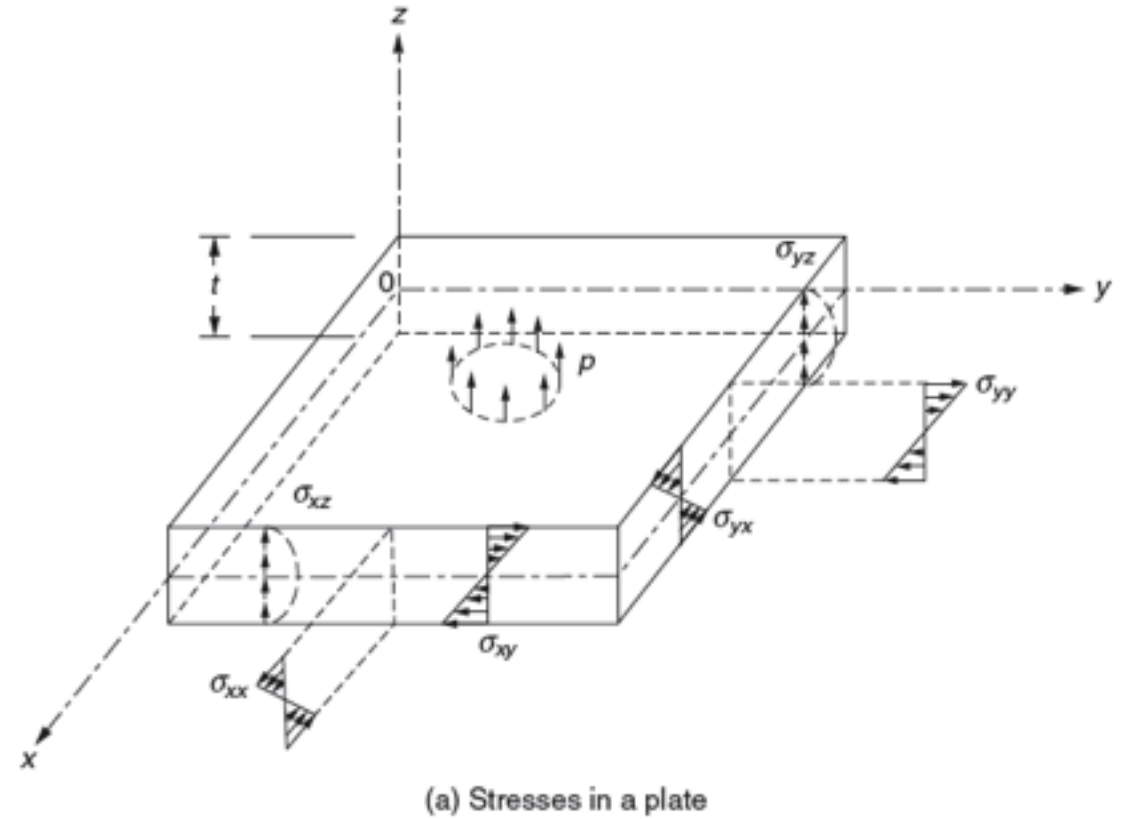
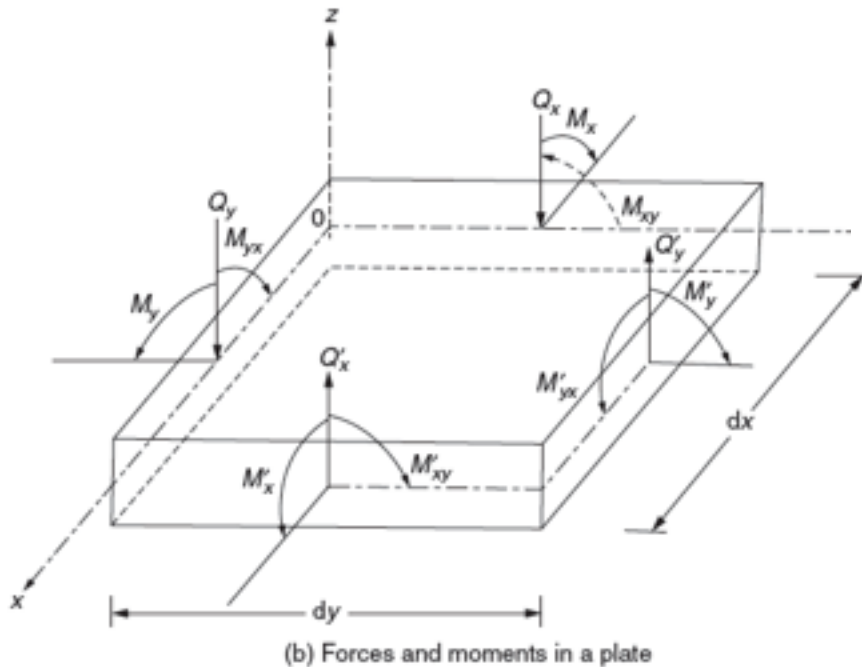
# CHARACTERISTICS

- displacement model should be continuous
- why is this a constant strain triangle?

# PLATE BENDING

# PLATE BENDING BEHAVIOUR

- thickness is small as compared to other dimensions
- deflections are small
- mid-plane does not undergo in-plane deformation
- transverse shear deformation is zero



# FINITE ELEMENT ANALYSIS OF PLATE BENDING

- special elements - transverse deflection of middle surface only
- other elements - include transverse shear
  
- displacement model
  - 'w' should be continuous
  - derivative should be continuous
  - for convergence, assumed displacement model will have constant curvature states

# TRIANGULAR PLATE BENDING ELEMENT

9 displacement degrees of freedom

Displacement model:

$$w(x, y) = a_1 + a_2x + a_3y + a_4x^2 + a_5xy + a_6y^2 + a_7x^3 + a_8(x^2y + xy^2) + a_9y^3$$

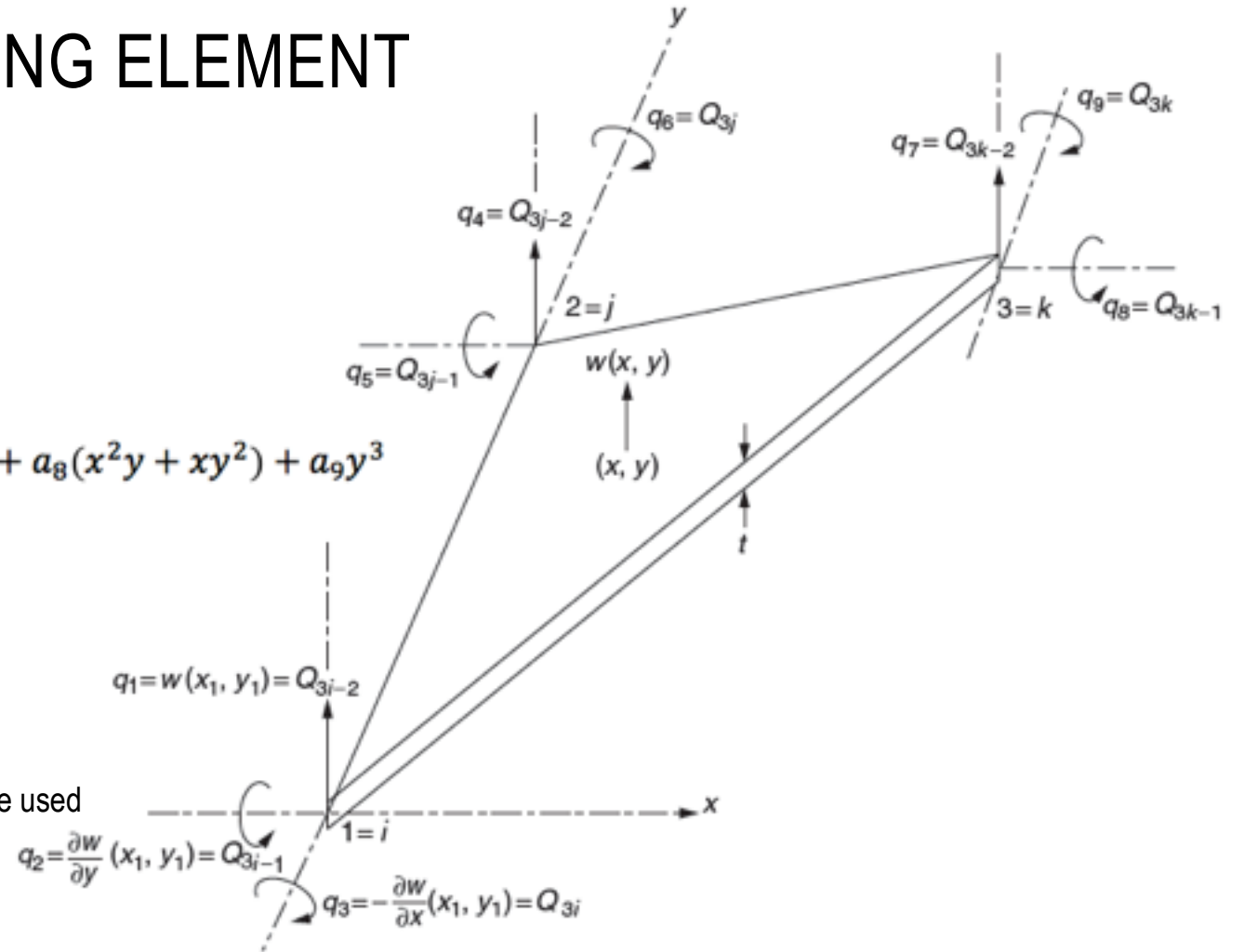
The same process is followed

Differences?

B matrix is a function of x, y and z!

thickness t cannot be assumed to be constant, so integral must be used

Transformation matrix will be 9x9



# HOMEWORK

- Check blackboard for practice problems on membrane and bending plates
- Answer Self-Check questions and discuss on the forum
- Continue working on Homework assignment 2.



# NEXT WEEK...

- Post processing
- Convergence
- Strain energy error