# Nonlinear Theory of Elasticity 

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## geometry description

Cartesian global coordinate system with base vectors of the Euclidian space


- orthonormal basis
- origin $O$
- point $P$
- domain $\Omega$ of a deformable body
- closed domain surface $\partial \Omega$

$$
\mathbf{e}_{i}:=\frac{\partial \mathbf{x}}{\partial x_{i}} \in \mathbb{R}^{3}, \quad \mathbf{x}_{i}^{T} \mathbf{x}_{j}=\delta_{i j}
$$



$$
\begin{aligned}
\mathbf{x} & =x_{1} \mathbf{e}_{1}+x_{2} \mathbf{e}_{2}+x_{3} \mathbf{e}_{3} \\
\mathbf{n} & =n_{1} \mathbf{e}_{1}+n_{2} \mathbf{e}_{2}+n_{3} \mathbf{e}_{3}
\end{aligned}
$$

solid body
rigid body
deformable body

## geometry description

neighboring points remain neighboring points independent of time
distant between points remains constant during displacement
distance between neighboring points may change with time

reference configuration $\Omega$

- often: state at time $t=0$
- material points $P(\mathbf{x}, t)$
instant configuration $\hat{\Omega}$
- often: state at time $\hat{t} \neq t$
- material points $P(\hat{\mathbf{x}}, t)$


$$
\begin{aligned}
\mathbf{u} & :=\hat{\mathbf{x}}-\mathbf{x}
\end{aligned} \quad \mathbf{x}=x_{1} \mathbf{e}_{1}+x_{2} \mathbf{e}_{2}+x_{3} \mathbf{e}_{3}, ~\left[\begin{array}{l}
u_{1} \\
u_{2} \\
u_{3}
\end{array}\right] \quad=\left[\begin{array}{l}
\hat{x}_{1} \\
\hat{x}_{2} \\
\hat{x}_{3}
\end{array}\right]-\left[\begin{array}{l}
x_{1} \\
x_{2} \\
x_{3}
\end{array}\right] \quad \hat{\mathbf{x}}=\hat{x}_{1} \mathbf{e}_{1}+\hat{x}_{2} \mathbf{e}_{2}+\hat{x}_{3} \mathbf{e}_{3} .
$$


displacement $\mathbf{u}$ is a combination of

- rigid body movement/rotation ( $A B=$ const for any two points)
- deformation ( $\mathrm{AB} \neq$ const)

A, B neighboring points of the deformable body

## kinematics - deformation state



- infinitesimal volume considered
- base vectors of reference and instant configuration
$d x_{i}$ : infinitesimal edge length

$$
\begin{aligned}
d \mathbf{x} & =\mathbf{e}_{1} d x_{1}+\mathbf{e}_{2} d x_{2}+\mathbf{e}_{3} d x_{3} \\
d \hat{\mathbf{x}} & =\mathbf{b}_{1} d x_{1}+\mathbf{b}_{2} d x_{2}+\mathbf{b}_{3} d x_{3}
\end{aligned}
$$

$\mathbf{e}_{i} \quad:=\frac{\partial \mathbf{x}}{\partial x_{i}} \quad i \in\{1,2,3\}$

$$
\mathbf{b}_{i}:=\frac{\partial \hat{\mathbf{x}}}{\partial x_{i}}=\frac{\partial(\mathbf{x}+\mathbf{u})}{\partial x_{i}}=\mathbf{e}_{i}+\frac{\partial \mathbf{u}}{\partial x_{i}}
$$

## kinematics - deformation state



## method of Lagrange

- material particle identification in the reference configuration $\Omega$
- particle location $\mathbf{x}$ at time $\hat{t}$ is a function of $(\mathbf{x}, t)$ and $\hat{t}$
- analog for state variables


## kinematics - deformation state



## method of Euler

- material particle identification in the instant configuration $\hat{\Omega}$
- particle location $\mathbf{x}$ at time $\hat{t}$ is a function of $(\hat{\mathbf{x}}, t)$ and $\hat{t}$
- analog for state variables


## kinematics - deformation gradient $\mathbf{F}$

- material deformation gradient $\mathbf{F}$
- representation of diagonal $d \hat{\mathbf{x}}$ as function of diagonal $d \mathbf{x}$
- columns of $\mathbf{F}$ are the instant base vectors $\mathbf{b}_{\mathrm{k}}(\mathrm{k}=1,2,3)$

$$
\begin{aligned}
d \hat{\mathbf{x}} & =\mathbf{F} d \mathbf{x} \\
{\left[\begin{array}{l}
d \hat{x}_{1} \\
d \hat{x}_{2} \\
d \hat{x}_{3}
\end{array}\right] } & =\left[\begin{array}{lll}
\frac{\partial \hat{x}_{1}}{\partial x_{1}} & \frac{\partial \hat{x}_{1}}{\partial x_{2}} & \frac{\partial \hat{x}_{1}}{\partial x_{3}} \\
\frac{\hat{x}_{2}}{\partial x_{1}} & \frac{\partial \hat{x}_{2}}{\partial x_{2}} & \frac{\partial \hat{x}_{2}}{\partial x_{3}} \\
\frac{\partial \hat{x}_{3}}{\partial x_{1}} & \frac{\partial \hat{x}_{3}}{\partial x_{2}} & \frac{\partial \hat{x}_{3}}{\partial x_{3}}
\end{array}\right]\left[\begin{array}{l}
d x_{1} \\
d x_{2} \\
d x_{3}
\end{array}\right] \\
& =\left[\begin{array}{lll}
\mathbf{b}_{1} & \mathbf{b}_{2} & \mathbf{b}_{3}
\end{array}\right]\left[\begin{array}{l}
d x_{1} \\
d x_{2} \\
d x_{3}
\end{array}\right]
\end{aligned}
$$

## kinematics - deformation gradient $\mathbf{F}$

- use the deformation gradient $\mathbf{F}$ to show the change in volume for the infinitesimal volume in instant and reference configuration



## kinematics - displacement gradient H

- split of the deformation gradient $\mathbf{F}$ into a unit matrix I and a matrix $\mathbf{H}$
- H contains the partial derivatives of u w.r.t. coordinates of ref. config.

$$
\begin{aligned}
& d \hat{\mathbf{x}}=(\mathbf{I}+\mathbf{H}) d \mathbf{x} \\
& {\left[\begin{array}{l}
d \hat{x}_{1} \\
d \hat{x}_{2} \\
d \hat{x}_{3}
\end{array}\right] }=\left[\begin{array}{ccc}
1+\frac{\partial u_{1}}{\partial x_{1}} & \frac{\partial u_{1}}{\partial x_{2}} & \frac{\partial u_{1}}{\partial x_{3}} \\
\frac{\partial u_{2}}{\partial x_{1}} & 1+\frac{\partial u_{2}}{\partial x_{2}} & \frac{\partial u_{2}}{\partial x_{3}} \\
\frac{\partial u_{3}}{\partial x_{1}} & \frac{\partial u_{3}}{\partial x_{2}} & 1+\frac{\partial u_{3}}{\partial x_{3}}
\end{array}\right]\left[\begin{array}{l}
d x_{1} \\
d x_{2} \\
d x_{3}
\end{array}\right] \\
& \text { with } \quad \mathbf{H}=\left[\begin{array}{lll}
\frac{\partial u_{1}}{\partial x_{1}} & \frac{\partial u_{1}}{\partial x_{2}} & \frac{\partial u_{1}}{\partial x_{3}} \\
\frac{\partial u_{2}}{\partial x_{1}} & \frac{\partial u_{2}}{\partial x_{2}} & \frac{\partial u_{2}}{\partial x_{3}} \\
\frac{\partial u_{3}}{\partial x_{1}} & \frac{\partial u_{3}}{\partial x_{2}} & \frac{\partial u_{3}}{\partial x_{3}}
\end{array}\right]
\end{aligned}
$$

## kinematics - state of strain

- consider the change the length of $d \mathbf{x}$
- deformation measure referred to the reference configuration
- results in the strain tensor of Green

$$
\begin{aligned}
d \hat{\mathbf{x}}^{T} d \hat{\mathbf{x}}-d \mathbf{x}^{T} d \mathbf{x} & =d \mathbf{x}^{T}(\mathbf{I}+\mathbf{H})^{T}(\mathbf{I}+\mathbf{H}) d \mathbf{x}-d \mathbf{x}^{T} d \mathbf{x} \\
& =d \mathbf{x}^{T}\left(\mathbf{H}+\mathbf{H}^{T}+\mathbf{H}^{T} \mathbf{H}\right) d \mathbf{x} \\
(\mathbf{H} & \left.+\mathbf{H}^{T}+\mathbf{H}^{T} \mathbf{H}\right):=2 \mathbf{E}
\end{aligned}
$$

$$
\begin{aligned}
\text { strain tensor of } \hat{\Omega}: \mathbf{E}= & \frac{1}{2}\left(\mathbf{H}+\mathbf{H}^{T}+\mathbf{H}^{T} \mathbf{H}\right) \\
\text { tensor coordinates: } e_{i m}= & \frac{1}{2}\left(\frac{\partial u_{i}}{\partial x_{m}}+\frac{\partial u_{m}}{\partial x_{i}}+\sum_{k} \frac{\partial u_{k}}{\partial x_{i}} \frac{\partial u_{k}}{\partial x_{m}}\right) \\
& \quad i, m, k \in\{1,2,3\}
\end{aligned}
$$

## kinematics - state of strain

- consider the change the length of $d \mathbf{x}$
- deformation measure referred to the reference configuration
- results in the strain tensor of Green-Lagrange

$$
\begin{aligned}
d \hat{\mathbf{x}}^{T} d \hat{\mathbf{x}}-d \mathbf{x}^{T} d \mathbf{x} & =d \mathbf{x}^{T}(\mathbf{I}+\mathbf{H})^{T}(\mathbf{I}+\mathbf{H}) d \mathbf{x}-d \mathbf{x}^{T} d \mathbf{x} \\
& =d \mathbf{x}^{T}\left(\mathbf{H}+\mathbf{H}^{T}+\mathbf{H}^{T} \mathbf{H}\right) d \mathbf{x} \\
(\mathbf{H} & \left.+\mathbf{H}^{T}+\mathbf{H}^{T} \mathbf{H}\right):=2 \mathbf{E}
\end{aligned}
$$

$$
\text { strain tensor of } \hat{\Omega}: \mathbf{E}=\frac{1}{2}\left(\mathbf{H}+\mathbf{H}^{T}+\mathbf{H}^{T} \mathbf{H}\right)
$$

$$
\begin{gathered}
\text { tensor coordinates: } e_{i m}=\frac{1}{2}\left(\frac{\partial u_{i}}{\partial x_{m}}+\frac{\partial u_{m}}{\partial x_{i}}+\sum_{k} \frac{\partial u_{k}}{\partial x_{i}} \frac{\partial u_{k}}{\partial x_{m}}\right) \\
\text { LINEAR THEORY } \\
i, m, k \in\{1,2,3\}
\end{gathered}
$$

## kinematics - Green strain tensor

strain tensor of $\hat{\Omega}: \mathbf{E}=\frac{1}{2}\left(\mathbf{H}+\mathbf{H}^{T}+\mathbf{H}^{T} \mathbf{H}\right)$
tensor coordinates: $e_{i m}=\frac{1}{2}\left(\frac{\partial u_{i}}{\partial x_{m}}+\frac{\partial u_{m}}{\partial x_{i}}+\sum_{k} \frac{\partial u_{k}}{\partial x_{i}} \frac{\partial u_{k}}{\partial x_{m}}\right)$ $i, m, k \in\{1,2,3\}$

$$
\mathbf{E}=\left[\begin{array}{lll}
e_{11} & e_{12} & e_{13} \\
e_{21} & e_{22} & e_{23} \\
e_{31} & e_{32} & e_{33}
\end{array}\right]
$$

- diagonal coefficients $e_{i i}$
- off-diagonal coefficients $e_{i m}$
stretch: measure of fibre elongation shear: measure of the angle between fibre angles


## kinematics - strain-displacement relation

... applying Voigt notation

$$
\begin{aligned}
\boldsymbol{\epsilon} & =\mathbf{D} \mathbf{u} \\
{\left[\begin{array}{l}
\epsilon_{11} \\
\epsilon_{22} \\
\epsilon_{33} \\
\epsilon_{23} \\
\epsilon_{31} \\
\epsilon_{12}
\end{array}\right] } & =\left[\begin{array}{ccc}
\frac{\partial}{\partial x_{1}} & 0 & 0 \\
0 & \frac{\partial}{\partial x_{2}} & 0 \\
0 & 0 & \frac{\partial}{\partial x_{3}} \\
0 & \frac{\partial}{\partial x_{3}} & \frac{\partial}{\partial x_{2}} \\
\frac{\partial}{\partial x_{3}} & 0 & \frac{\partial}{\partial x_{1}} \\
\frac{\partial}{\partial x_{2}} & \frac{\partial}{\partial x_{1}} & 0
\end{array}\right]\left[\begin{array}{l}
u_{1} \\
u_{2} \\
u_{3}
\end{array}\right] \\
\epsilon_{i i}=e_{i i} & =\left(\frac{\partial u_{i}}{\partial x_{i}}+\frac{1}{2} \sum_{k} \frac{\partial u_{k}}{\partial u_{i}} \frac{\partial u_{k}}{\partial u_{i}}\right) \\
\epsilon_{i j}=2 e_{i j} & =\left(\frac{\partial u_{i}}{\partial x_{j}}+\frac{\partial u_{j}}{\partial x_{i}}+\sum_{k} \frac{\partial u_{k}}{\partial u_{i}} \frac{\partial u_{k}}{\partial u_{j}}\right)
\end{aligned}
$$

stress vector - stress tensor

- stress vector in direction of the surface normal $\mathbf{n}$
- action of subfield $C_{1}$ on $C_{2}$ is replaced by a fictitious force $d p$


$$
\begin{aligned}
\hat{\mathbf{t}} & =\lim _{d a \rightarrow 0} \frac{d \mathbf{p}}{d a} \\
{\left[\begin{array}{c}
\hat{t}_{1} \\
\hat{t}_{2} \\
\hat{t}_{3}
\end{array}\right] } & =\hat{t}_{1} \mathbf{e}_{1}+\hat{t}_{2} \mathbf{e}_{2}+\hat{t}_{3} \mathbf{e}_{3}
\end{aligned}
$$

$$
\text { stress vector at point } \hat{P}
$$

## statics - stress tensor of Cauchy



- stress tensor of the deformed configuration
- columns of the Cauchy stress tensor are the stress vectors on the positive faces of the element


forces in $x_{2^{-}}$and $x_{3}$-direction analogous load acting on the unit volume $\rightarrow \hat{\rho} \mathbf{p}$

$$
\sum F=\mathbf{0}=-\hat{\mathbf{s}}_{1} d \hat{x}_{2} d \hat{x}_{3}+\left(\hat{\mathbf{s}}_{1}+\frac{\partial \hat{\mathbf{s}}_{1}}{\partial \hat{x}_{1}} d \hat{x}_{1}\right) d \hat{x}_{2} d \hat{x}_{3}+\hat{\rho} \mathbf{p} d \hat{x}_{1} d \hat{x}_{2} d \hat{x}_{3}
$$

# statics - equilibrium 


sum of the moments acting on the element is null in the state of equilibrium
$\rightarrow$ from this follows the symmetry of the Cauchy stress tensor

$$
\hat{s}_{i k}=\hat{s}_{k i}
$$

## statics - 1st Piola-Kirchhoff stress tensor

- consider a surface element of the reference configuration $\Omega$ which is replaced to the instant configuration $\hat{\Omega}$

$$
d \mathbf{a}(=\mathbf{n} d a) \quad \rightarrow \quad d \hat{\mathbf{a}}(=\mathbf{n} d \hat{a})
$$

- $1^{\text {st }}$ Piola-Kirchhoff tensor causes the same force $d \mathbf{f}$ (definition!)

$$
\text { on } d \mathbf{a}(=\mathbf{n} d a) \& d \hat{\mathbf{a}}(=\mathbf{n} d \hat{a})
$$

$$
d \mathbf{f}=\quad \mathbf{P} d \mathbf{a}=\hat{\mathbf{S}} d \hat{\mathbf{a}}=(\operatorname{det} \mathbf{F}) \hat{\mathbf{S}} \hat{\mathbf{F}}^{T} d \mathbf{a}
$$

$$
\mathbf{P}=(\operatorname{det} \mathbf{F}) \hat{\mathbf{S}} \hat{\mathbf{F}}^{T}
$$

$1^{\text {st }}$ Piola Kirchhoff tensor

- stress coordinates are referred to the global base vectors
- $1^{\text {st }}$ PK stress tensor is unsymmetric, in general, not in use!


## statics - 2nd Piola-Kirchhoff stress tensor

- PK1 force vector referred to the basis of the instant configuration $\hat{\Omega}$

$$
\begin{aligned}
\mathbf{p}_{k} & =p_{1 k} \mathbf{e}_{1}+p_{2 k} \mathbf{e}_{2}+p_{3 k} \mathbf{e}_{3} \\
\mathbf{p}_{k} & =s_{1 k} \mathbf{b}_{1}+s_{2 k} \mathbf{b}_{2}+s_{3 k} \mathbf{b}_{3}
\end{aligned}
$$

- bases vectors in $\hat{\Omega}$ are the columns of the deformation gradient

$$
\mathbf{P}=\mathbf{F S}
$$

- relation between Cauchy stress tensor and $2^{\text {nd }} \mathrm{PK}$ tensor

$$
\begin{aligned}
\mathbf{S} & =(\operatorname{det} \mathbf{F}) \hat{\mathbf{F}} \hat{\mathbf{S}} \hat{\mathbf{F}}^{t} \\
\hat{\mathbf{S}} & =(\operatorname{det} \hat{\mathbf{F}}) \mathbf{F} \mathbf{S} \mathbf{F}^{t}
\end{aligned}
$$

$2^{\text {nd }}$ Piola Kirchhoff tensor is symmetric! energetically conjugate stress tensor to the Green strain tensor
statics - stress-strain relation
... linear elasticity - Hooke's law

$$
\begin{aligned}
& \boldsymbol{\sigma}=\mathrm{C} \boldsymbol{\epsilon} \\
& {\left[\begin{array}{l}
\sigma_{11} \\
\sigma_{22} \\
\sigma_{33} \\
\sigma_{23} \\
\sigma_{31} \\
\sigma_{12}
\end{array}\right]=\left[\begin{array}{llllll}
a & b & b & 0 & 0 & 0 \\
b & a & b & 0 & 0 & 0 \\
b & b & a & 0 & 0 & 0 \\
0 & 0 & 0 & c & 0 & 0 \\
0 & 0 & 0 & 0 & c & 0 \\
0 & 0 & 0 & 0 & 0 & c
\end{array}\right]\left[\begin{array}{l}
\epsilon_{11} \\
\epsilon_{22} \\
\epsilon_{33} \\
\epsilon_{23} \\
\epsilon_{31} \\
\epsilon_{12}
\end{array}\right]} \\
& a=\frac{E(1-\nu)}{(1+\nu)(1-2 \nu)} \\
& b=a \frac{\nu}{(1-\nu)} \quad c=a \frac{(1-2 \nu)}{2(1-\nu)}
\end{aligned}
$$

## partial differential equation

## governing equations

$$
\begin{array}{lll}
\text { strain }-\operatorname{displm} . & e_{i m}=\frac{1}{2}\left(u_{i, m}+u_{m, i}+\sum_{k} u_{k, m} u_{k, i}\right) & \hat{\mathbf{x}} \in \hat{\Omega} \\
\text { stress }- \text { strain } & \hat{s}_{i m}=C_{i m k l} e_{k l} & \hat{\mathbf{x}} \in \hat{\Omega} \\
\text { equilibrium } & 0 & =\hat{s}_{i 1,1}+\hat{s}_{i 2,2}+\hat{s}_{i 3,3}+\hat{\rho} q_{i} \\
\text { stress vector } & \hat{t}_{i}=\hat{\mathbf{s}}_{i}^{T} \mathbf{n} & \hat{\mathbf{x}} \in \hat{\Omega} \\
\text { n } & \hat{\mathbf{x}} \in \partial \hat{\Omega}
\end{array}
$$

$e_{i m} / \hat{s}_{i m} \quad$ Green-Lagrange/Cauchy strain/stress tensor coordinates

Dirichlet (prescribed displacements)

$$
\mathbf{x} \in \partial \hat{\Omega} \wedge \mathbf{u} \in \hat{\Gamma}_{u}: u_{i}=u_{i 0}
$$

Neumann (prescribed stresses)

$$
\mathbf{x} \in \partial \hat{\Omega} \wedge \mathbf{t} \in \hat{\Gamma}_{t}: t_{i}=t_{i 0}
$$

$\hat{\Gamma}_{u}$ : boundary of prescribed displacements components
$\hat{\Gamma}_{t}$ : boundary of prescribed stress vector components

## solution approach - FEM

weighted residual approach, cf linear theory of elasticity

- choice of a suited approximation rule for the displacement state
- definition of residuals which are not a priori satisfied
- choose of admissible/suited weight functions
- here: Bubnov-Galerkin approach: variation of displacements
- multiply residuals with weight functions
- integration over volume of the instant configuration

$$
\int_{\hat{\Omega}} g(\hat{\mathbf{x}}) r(\hat{\mathbf{x}}) d \hat{\mathbf{x}}=0
$$

## spatial integral form

$1^{\text {st }}$ integral form

$$
\begin{aligned}
& \int_{\hat{\Omega}} \sum_{i} \sum_{m}\left(\delta u_{i} \frac{\partial \hat{s}_{i m}}{\partial \hat{x}_{m}}\right) d \hat{v}+\int_{\hat{\Omega}} \sum_{i} \delta u_{i} \hat{\rho} q_{i} d \hat{v}+ \\
& \int_{\delta \hat{\Omega}} \sum_{i} \delta u_{i}\left(\hat{t}_{i}-\sum_{m} \hat{s}_{i m} n_{m}\right) d \hat{a}+ \\
& \quad \int_{\hat{\Gamma}_{t}} \sum_{i} \delta u_{i}\left(\hat{t}_{i}-\hat{t}_{i_{0}}\right) d \hat{a}+\int_{\hat{\Gamma}_{u}} \sum_{i} \delta \hat{t}_{i}\left(\hat{u}_{i}-\hat{u}_{i_{0}}\right) d \hat{a}=0
\end{aligned}
$$

$2^{\text {nd }}$ integral form (Principle of virtual work)

$$
\begin{gathered}
\int_{\hat{\Omega}} \sum_{i} \sum_{m} \hat{s}_{i m} \delta\left(\frac{\partial u_{i}}{\partial \hat{x}_{m}}\right) d \hat{v}=\int_{\hat{\Omega}} \sum_{i} \delta u_{i} \hat{\rho} q_{i} d \hat{v}+\int_{\hat{\Gamma}_{t}} \sum_{i} \delta u_{i} \hat{t}_{i_{0}} d \hat{a} \\
u_{i}=u_{i_{0}} \quad \hat{x}_{i} \in \hat{\Gamma}_{u}
\end{gathered}
$$

- spatial integral form derived for volume elements of the instant config.
- volume of the body in $\hat{\Omega}$ is unknown!


## material integral form

integral equation is referred to the known reference configuration
replace ...

- unknow volume $d \hat{v} \quad$ with known volume $d v$
- Cauchy coordinates $\hat{s}_{i m}$ with $2^{\text {nd }}$ Piola-Kirchhoff coordinates $s_{i m}$
- instant coordinate $\hat{x}_{i}$ with $x_{i}+u_{i}$
on the left hand follows

$$
\sum_{i} \sum_{m} \hat{s}_{i m} \delta\left(\frac{\partial u_{i}}{\partial \hat{x}_{m}}\right) d \hat{v}=\sum_{i} \sum_{m} \delta e_{i m} s_{i m} d v
$$

on the right hand follows in analogy

$$
\sum_{i} \delta u_{i} \hat{t}_{i} d \hat{a}=\sum_{i} \delta u_{i} p_{i} d a
$$

## material integral form

## Principle of virtual work

$$
\begin{gathered}
\int_{\Omega} \sum_{i} \sum_{m} \delta e_{i m} s_{i m} d v=\int_{\Omega} \sum_{i} \delta u_{i} q_{i} d v+ \\
\int_{\Gamma_{t}} \sum_{i} \delta u_{i} p_{i 0} d a \\
\wedge u_{i}=u_{i 0} \quad \mathbf{x} \in \Gamma_{u}
\end{gathered}
$$

- strains are nonlinear functions of the derivatives (Green-Lagrange)
- stresses ( $2^{\text {nd }} \mathrm{PK}$ ) are referred to base vectors of reference $\&$ instant config.
- conservative loads are assumed $\rightarrow$ independent of the displacements

- stepwise solution for the nonlinear equations $0, \Delta t, 2 \Delta t, \ldots, t$
- initial configuration is assumed to be known
- solution at the end of each step
- governing equations are incremental equations
- consistent linearization leads to incremental equations

incremental equations
$\Omega \quad$ reference configuration
$\hat{\Omega} \quad$ instant configuration I, known from previous step
$\bar{\Omega} \quad$ instant configuration II, unknown
$\overline{\mathbf{u}}$ unknown displacement state at the end of step $i$
u known displacement state at beginning of step $i$
$\Delta \mathbf{u} \quad$ displacement increment from $\hat{\Omega}$ to $\bar{\Omega}$


Total Lagrangian (TL) formulation
$\rightarrow$ referred to $\Omega$
Updated Lagrangian (UL) formulation
$\rightarrow$ referred to $\hat{\Omega}$

## incremental equations

## state variables

$$
\begin{aligned}
\bar{u}_{i} & =u_{i}+\Delta u_{i} \\
\bar{q}_{i 0} & =q_{i 0}+\Delta q_{i 0} \\
\bar{e}_{i j} & =e_{i j}+\Delta e_{i j} \\
\bar{s}_{i j} & =s_{i j}+\Delta s_{i j}
\end{aligned}
$$

Incremental strain-displacement relationship

$$
\begin{aligned}
& \bar{e}_{i j}= \frac{1}{2}\left(\bar{u}_{i, j}+\bar{u}_{j, i}+\sum_{k=1}^{3} \bar{u}_{k, i} \bar{u}_{k, j}\right) \\
&= \frac{1}{2}\left(u_{i, j}+\Delta u_{i, j}+u_{j, i}+\Delta u_{j, i}+\sum_{k=1}^{3}\left(u_{k, i}+\Delta u_{k, i}\right)\left(u_{k, j}+\Delta u_{k, j}\right)\right) \\
&= \frac{1}{2}\left(u_{i, j}+u_{j, i}+\Delta u_{i, j}+\Delta u_{j, i}+\right. \\
&\left.\quad \sum_{k=1}^{3} u_{k, i} u_{k, j}+u_{k, i} \Delta u_{k, j}+u_{k, j} \Delta u_{k, i}+\Delta u_{k, i} \Delta u_{k, j}\right)
\end{aligned}
$$

## incremental equations

## state variables

$$
\begin{aligned}
\bar{u}_{i} & =u_{i}+\Delta u_{i} \\
\bar{q}_{i 0} & =q_{i 0}+\Delta q_{i 0} \\
\bar{e}_{i j} & =e_{i j}+\Delta e_{i j} \\
\bar{s}_{i j} & =s_{i j}+\Delta s_{i j}
\end{aligned}
$$

Incremental strain-displacement relationship

$$
\begin{aligned}
\bar{e}_{i j} & =e_{i j}+\Delta e_{i j}^{L}+\Delta e_{i j}^{N} \\
e_{i j} & =\frac{1}{2}\left(u_{i, j}+u_{j, i}+\sum_{k=1}^{3} u_{k, i} u_{k, j}\right) \\
\Delta e_{i j}^{L} & =\frac{1}{2}\left(\Delta u_{i, j}+\Delta u_{j, i}+\sum_{k=1}^{3}\left(u_{k, i} \Delta u_{k, j}+u_{k, j} \Delta u_{k, i}\right)\right) \\
\Delta e_{i j}^{N} & =\frac{1}{2} \sum_{k=1}^{3}\left(\Delta u_{k, i} \Delta u_{k, j}\right)
\end{aligned}
$$

## incremental equations

## state variables

$$
\begin{aligned}
\bar{u}_{i} & =u_{i}+\Delta u_{i} \\
\bar{q}_{i 0} & =q_{i 0}+\Delta q_{i 0} \\
\bar{e}_{i j} & =e_{i j}+\Delta e_{i j} \\
\bar{s}_{i j} & =s_{i j}+\Delta s_{i j}
\end{aligned}
$$

Variation of the state of displacements

$$
\begin{aligned}
\delta \overline{\mathbf{u}} & =\delta(\mathbf{u}+\Delta \mathbf{u}) \\
& =\delta \mathbf{u}+\delta(\Delta \mathbf{u}) \\
& =\delta(\Delta \mathbf{u})
\end{aligned}
$$

Variation of the state of strain

$$
\begin{aligned}
\delta \bar{e}_{i j} & =\delta\left(\Delta e_{i j}\right) \\
& =\delta\left(\Delta e_{i j}^{L}\right)+\delta\left(\Delta e_{i j}^{N}\right) \\
\delta\left(\Delta e_{i j}^{N}\right) & =\frac{1}{2} \sum_{k=1}^{3}\left(\Delta u_{k, i} \delta\left(\Delta u_{k, j}\right)+\Delta u_{k, j} \delta\left(\Delta u_{k, i}\right)\right)
\end{aligned}
$$

## incremental equations

Governing equations in vector notation

$$
\begin{aligned}
& \int_{\Omega} \sum_{i} \sum_{j} \delta\left(\Delta e_{i j}^{L}\right) \Delta s_{i j} d v+\int_{\Omega} \sum_{i} \sum_{j} \delta\left(\Delta e_{i j}^{N}\right) s_{i j} d v= \\
& \Delta r+\int_{\Omega} \sum_{i} \delta\left(\Delta u_{i}\right) \Delta p_{i} \rho d v+\int_{\Gamma_{t}} \sum_{i} \delta\left(\Delta u_{i}\right) \Delta t_{i 0} d a
\end{aligned}
$$

with

$$
\begin{aligned}
\Delta r= & \int_{\Omega} \sum_{i} \delta\left(\Delta u_{i}\right) p_{i} \rho d v+\int_{\Gamma_{t}} \sum_{i} \delta\left(\Delta u_{i}\right) t_{i 0} d a \\
& -\int_{\Omega} \sum_{i} \sum_{j} \delta\left(\Delta e_{i j}^{L}\right) s_{i j} d v
\end{aligned}
$$

## incremental equations

Governing equations in vector notation

$$
\begin{gathered}
\int_{\Omega} \delta\left(\Delta \boldsymbol{\epsilon}_{C}\right)^{T} \mathbf{C}\left(\Delta \boldsymbol{\epsilon}_{C}\right) d v+\int_{\Omega} \delta\left(\Delta \boldsymbol{\epsilon}_{L}\right)^{T} \mathbf{C}\left(\Delta \boldsymbol{\epsilon}_{L}\right) d v+\int_{\Omega} \sum_{k} \delta\left(\Delta \mathbf{g}_{k}\right)^{T} \mathbf{S} \Delta \mathbf{g}_{k} d v \\
=\Delta r+\int_{\Omega} \delta(\Delta \mathbf{u})^{T} \Delta \mathbf{p} \rho d v+\int_{\Gamma_{t}} \delta(\Delta \mathbf{u})^{T} \Delta \mathbf{t}_{0} d a
\end{gathered}
$$

with

$$
\Delta r=\int_{\Omega} \delta(\Delta \mathbf{u})^{T} \mathbf{p} \rho d v+\int_{\Gamma_{t}} \delta(\Delta \mathbf{u})^{T} \mathbf{t}_{0} d a-\int_{\Omega} \delta\left(\Delta \boldsymbol{\epsilon}_{C}+\Delta \boldsymbol{\epsilon}_{L}\right)^{T} \boldsymbol{\sigma} d v
$$

## Governing equations in vector notation

$$
\begin{aligned}
& \Delta \boldsymbol{\epsilon}_{C}=\left[\begin{array}{l}
\Delta e_{11_{C}} \\
\Delta e_{22_{C}} \\
\Delta e_{33_{C}} \\
\Delta e_{12_{C}} \\
\Delta e_{23_{C}} \\
\Delta e_{31_{C}}
\end{array}\right]=\frac{1}{2}\left[\begin{array}{c}
2 \Delta u_{1,1} \\
2 \Delta u_{2,2} \\
2 \Delta u_{3,3} \\
\left(\Delta u_{1,2}+\Delta u_{2,1}\right) \\
\left(\Delta u_{2,3}+\Delta u_{3,2}\right) \\
\left(\Delta u_{3,1}+\Delta u_{1,3}\right)
\end{array}\right] \\
& \Delta \boldsymbol{\epsilon}_{L}=\left[\begin{array}{l}
\Delta \mathbf{g}_{k}=\left[\begin{array}{l}
\Delta u_{k, 1} \\
\Delta u_{k, 2} \\
\Delta u_{k, 3}
\end{array}\right] \\
\Delta e_{11_{L}} \\
\Delta e_{22_{L}} \\
\Delta e_{33_{L}} \\
\Delta e_{12_{L}} \\
\Delta e_{23_{L}} \\
\Delta e_{31_{L}}
\end{array}\right]=\frac{1}{2}\left[\begin{array}{c}
\sum_{k=1}^{3} 2\left(u_{k, 1} \Delta u_{k, 1}\right) \\
\sum_{k=1}^{3} 2\left(u_{k, 2} \Delta u_{k, 2}\right) \\
\sum_{k=1}^{3} 2\left(u_{k, 3} \Delta u_{k, 3}\right) \\
\sum_{k=1}^{3}\left(u_{k, 1} \Delta u_{k, 2}+u_{k, 2} \Delta u_{k, 1}\right) \\
\sum_{k=1}^{3}\left(u_{k, 2} \Delta u_{k, 3}+u_{k, 3} \Delta u_{k, 2}\right) \\
\sum_{k=1}^{3}\left(u_{k, 3} \Delta u_{k, 1}+u_{k, 1} \Delta u_{k, 3}\right)
\end{array}\right]
\end{aligned}
$$

