# Bending Deflection Statically Indeterminate Beams 

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## Recap

## Procedure for Statically Indeterminate Problems

## I. Free Body Diagram

II. Equilibrium of Forces (and Moments)
III. Displacement Compatibility
IV. Force-Displacement (Stress-Strain) Relations

## Solve when number of equations = number of unknowns

V. Answer the Question! - Typically calculate desired internal stresses, relevant displacements, or failure criteria

> For bending, Force-Displacement relationships come from Moment-Curvature relationship
> (ie: use Method of Integration or Method of Superposition)

## Statically Indeterminate Beams

Many more redundancies are possible for beams:

- Draw FBD and count number of redundancies
- Each redundancy gives rise to the need for a compatibility equation

- 4 reactions
- 3 equilibrium equations
$4-3=1$
$1^{\text {st }}$ degree statically indeterminate

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## Statically Indeterminate Beams

Many more redundancies are possible for beams:

- Draw FBD and count number of redundancies
- Each redundancy gives rise to the need for a compatibility equation

- 6 reactions
- 3 equilibrium equations
$6-3=3$
3 ${ }^{\text {rd }}$ degree statically indeterminate

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# Solving statically indeterminate beams using method of integration 

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## What is the difference between a support and a force?



Displacement Compatibility
(support places constraint on deformation)

## Method of Integration



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## Method of Integration (cont)



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## Method of Integration (cont)


$v\left(z, V_{B}\right)$ How to determine $\mathrm{V}_{\mathrm{B}}$ ?


Compatibility $\mathrm{BC}: ~$ At $\mathrm{B}, \mathrm{v}=\mathrm{O} \longrightarrow$ Solve for $\mathrm{V}_{\mathrm{B}}$

# Compatibility equations for beams are simply the boundary conditions at redundant supports 

## Example 1

## Problem Statement

Determine deflection equation for the beam using method of integration:


## Solution

2) Equilibrium:
$\sum \vec{F} \Rightarrow H_{A}=0$
$\sum F^{\dagger} \Rightarrow V_{A}+V_{B}=q L$
$\sum M_{A} \Rightarrow M_{A}=L V_{B}-\frac{q L^{2}}{2}=\frac{q L^{2}}{2}-L V_{A}$
Treat reaction forces as knowns!

## Example 1


4) Determine moment equation:

$$
\begin{aligned}
\sum M_{z}^{c c w+} \Rightarrow M & =M_{A}+V_{A} z-(q z) \frac{z}{2} \\
& =M_{A}+V_{A} z-\frac{q}{2} z^{2}
\end{aligned}
$$



Can also use step function approach

$$
M=\overbrace{M_{A}[z-0]^{0}+V_{A}[z-0]-\frac{q}{2}[z-0]^{2}+V_{B}[z-L]}^{\text {always off }}
$$

## Example 1

5) Integrate Moment equation to get

$\int-E I v^{\prime \prime}=M_{A}+V_{A} z-\frac{q}{2} z^{2}=\mathrm{M}(\mathrm{z})$

$$
\begin{aligned}
& -E I v^{\prime}=M_{A} z+\frac{V_{A}}{2} z^{2}-\frac{q}{6} z^{3}+C_{1}=-\operatorname{EI} \theta(z) \\
& -E I v=\frac{M_{A}}{2} z^{2}+\frac{V_{A}}{6} z^{3}-\frac{q}{24} z^{4}+C_{1} z+C_{2}=-\operatorname{EIv}(z)
\end{aligned}
$$

We now have expressions for $v$ and $\mathrm{v}^{\prime}$, but need to determine constants of integration and unknown reactions

## Example 1

5a) Solve for Constants of Integration using $B C$ 's:

$$
-E I v^{\prime}=M_{A} z+\frac{V_{A}}{2} z^{2}-\frac{q}{6} z^{3}+\not_{1}^{0}=-E I \theta(z)
$$


$-E I v=\frac{M_{A}}{2} z^{2}+\frac{V_{A}}{6} z^{3}-\frac{q}{24} z^{4}+\not \varnothing_{1}^{0} z+C_{2}=-E \operatorname{Iv}(z)$

Boundary Conditions:

$$
\begin{gathered}
\text { At } \mathrm{z}=\mathrm{o}, \theta=\mathrm{o} \quad \Rightarrow-E I(0)=-\frac{q}{6}(0)^{3}+\frac{V_{A}}{2}(0)^{2}+M_{A}(0)+C_{1} \\
\therefore C_{1}=0
\end{gathered}
$$

## Example 1

5a) Solve for Constants of Integration
 using $B C$ 's:

$$
-E I v^{\prime}=M_{A} z+\frac{V_{A}}{2} z^{2}-\frac{q}{6} z^{3}+\not_{1}^{\prime 0}=-E I \theta(z)
$$



Fixed support
$-E I v=\frac{M_{A}}{2} z^{2}+\frac{V_{A}}{6} z^{3}-\frac{q}{24} z^{4}+\not \mathscr{C}_{1}^{0} z+\mathscr{C}_{2}^{0}=-\operatorname{EIv}(z)$

$$
\theta=\mathrm{o}, \mathrm{v}=\mathrm{o}
$$

Boundary Conditions:

$$
\text { At } \mathrm{z}=\mathrm{o}, \mathrm{v}=\mathrm{o} \quad \Rightarrow-E I(0)=-\frac{q}{24}(0)^{4}+\frac{V_{A}}{6}(0)^{3}+\frac{M_{A}}{2}(0)^{2}+C_{2}
$$

$$
\therefore C_{2}=0
$$

## Example 1

5b) Solve for Reaction Forces using BC's (imposed by redundant support):

$$
\begin{aligned}
& -E I v^{\prime}=M_{A} z+\frac{V_{A}}{2} z^{2}-\frac{q}{6} z^{3}=-\operatorname{EI\theta }(z) \\
& -E I v=\frac{M_{A}}{2} z^{2}+\frac{V_{A}}{6} z^{3}-\frac{q}{24} z^{4}=-\operatorname{EIv}(z)
\end{aligned}
$$


roller support

$$
\mathrm{v}=\mathrm{o}
$$

Boundary Conditions:

$$
\begin{aligned}
\text { At } \mathrm{z}=\mathrm{L}, \mathrm{v} & =\mathrm{o} \quad \Rightarrow-E I(0)=-\frac{q}{24}(L)^{4}+\frac{V_{A}}{6}(L)^{3}+\frac{M_{A}}{2}(L)^{2} \\
V_{A} & =\frac{q L}{4}-\frac{3 M_{A}}{L} \quad M_{A}=\frac{q L^{2}}{2}-L V_{A} \quad V_{B}=q L-V_{A}
\end{aligned}
$$

## Example 1

5b) Solve for Reaction Forces using


$$
V_{A}=\frac{q L}{4}-\frac{3 M_{A}}{L}, \quad M_{A}=\frac{q L^{2}}{2}-L V_{A}, \quad V_{B}=q L-V_{A}
$$

$$
V_{A}=\frac{5}{8} q L
$$

$$
M_{A}=-\frac{q L^{2}}{8}
$$

$$
V_{B}=\frac{3}{8} q L
$$



## Example 1

We were asked to determine deflection equation:

$$
\begin{aligned}
& -E I v=\frac{M_{A}}{2} z^{2}+\frac{V_{A}}{6} z^{3}-\frac{q}{24} z^{4} \\
& v=\frac{q z^{2}}{48 E I}\left(3 L^{2}-5 L z+2 z^{2}\right)
\end{aligned}
$$

Max Displacement:



$$
\begin{aligned}
& v^{\prime}=\frac{q}{48 E I}\left(6 L^{2} z-15 L z^{2}+8 z^{3}\right)=0 \quad \Rightarrow z \approx 0.5785 L \\
& v(0.5785 L)=0.005416 \frac{q L^{2}}{E I}
\end{aligned}
$$

## Example 1



Now that the reactions are known:

$$
\begin{aligned}
M(z) & =-E I v^{\prime \prime}=M_{A}+V_{A} z-\frac{q}{2} z^{2} \\
& =-\frac{q L^{2}}{8}+\frac{5 q L z}{8}-\frac{q z^{2}}{2} \\
V(z) & =-E I v^{\prime \prime \prime}=\frac{5 q L}{8}-q z
\end{aligned}
$$



## Solving statically indeterminate beams using superposition

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## Method of Superposition

Determine reaction forces:

2) Equilibrium:
$\sum \vec{F} \Rightarrow H_{A}=0$
$\sum F^{\uparrow} \Rightarrow V_{A}+V_{B}=P$


- 4 reactions
- 3 equilibrium equations
$1^{\text {st }}$ degree statically indeterminate
$\sum M_{A} \Rightarrow 3 L V_{B}=2 L P+M_{A}$
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## Method of Superposition (cont)

How do we get compatibility equation?


Split into two statically determinate problems


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## Method of Superposition (cont)

How do we get Force-Displacement relations?
We have been doing this in the previous lectures

Integrate Moment Curvature Relation

$$
M=-E I \frac{d^{2} v}{d z^{2}} \quad \begin{gathered}
\text { Can integrate } \\
\text { to find } v
\end{gathered}
$$

Standard Case Solutions


$$
\stackrel{+}{\theta_{B}^{+}}=\frac{P L^{2}}{2 E I} \quad \stackrel{\downarrow+}{v_{B}}=\frac{P L^{3}}{3 E I}
$$

## Method of Superposition (cont)

From the standard case:
4) Force-Displacement:


$$
\theta_{P}=\frac{P(2 L)^{2}}{2 E I}=\frac{2 P L^{2}}{E I}
$$

$$
v_{B 1}=v_{P}+\theta_{P} \cdot L
$$

$$
=\frac{P(2 L)^{3}}{3 E I}+\frac{2 P L^{2}}{E I} \cdot L
$$

$$
=\frac{14 P L^{3}}{3 E I}
$$

## Method of Superposition (cont)



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## Additional remarks about bending deflections

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## Remarks about Beam Deflections



Bending Deformation $=$ Shear Deformation + Moment Deformation THDelft

## Remarks about Beam Deflections



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## Remark about Beam Deflections

For bending deformation problems

## negligible

Deformation $=$ Axialmanation + Shearmation + Moment Deformation

BUT!
If moment deformation is not present, deformation is not negligible


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## Example 2

Calculate reaction forces at A and D:

First, can we see any simplifications?

## Symmetry!



Symmetry implies:

- Reactions at A = reactions at D
- Slope at symmetry plane $=0$
- Shear force at symmetry plane $=0$

We will solve using superposition and standard cases


## Example 2

Calculate reaction forces:
2) Equilibrium

$$
\begin{aligned}
& \sum \vec{F} \Rightarrow H_{A}=-H_{F} \\
& \sum F^{\uparrow+} \Rightarrow V_{A}=\frac{q L}{2} \\
& \sum M_{A}^{c c \omega+} \Rightarrow M_{A}+\frac{q L^{2}}{8}+H_{F} L=M_{F}
\end{aligned}
$$



Split problem into two: beam AB and beam BF

## Example 2

Calculate reaction forces:
3) Compatibility


$$
\theta_{F}=0, \underbrace{\mathrm{~V}_{B}=0} \text { (axial deformation negligible) }
$$

## Example 2

## 4a) Force-Displacement for Beam $A B$


$\overrightarrow{v_{B}^{+}}=\frac{H_{F} L^{3}}{3 E I}$
$\theta_{B}^{c w+}=\frac{H_{F} L^{2}}{2 E I}$
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$$
\vec{v}_{B}^{+}=-\frac{M_{F} L^{2}}{2 E I}+\frac{q L^{4}}{16 E I}
$$

$$
\theta_{B}^{c \text { c+ }}=-\frac{M_{F} L}{E I}+\frac{q L^{3}}{8 E I}
$$

## Example 2

4a) Force-Displacement for Beam $A B$


$$
\begin{aligned}
& \vec{v}_{B}^{+}=\frac{H_{F} L^{3}}{3 E I}-\frac{M_{F} L^{2}}{2 E I}+\frac{q L^{4}}{16 E I}=0 \\
& \theta_{B}^{c w+}=\frac{H_{F} L^{2}}{2 E I}-\frac{M_{F} L}{E I}+\frac{q L^{3}}{8 E I}
\end{aligned}
$$

Recall compatibility:
Still need $\theta_{\mathrm{F}} \longrightarrow \theta_{\mathrm{F}}=0, \quad \mathrm{v}_{B}=0$

## Example 2

4b) Force-Displacement for Beam BF

$+$

$\theta_{F}=0$

$$
\theta_{F}^{\text {ew+ }}=-\frac{M_{F} L}{E I}
$$

$$
\theta_{F}^{c w+}=\frac{q L^{3}}{48 E I}-\frac{M_{F} L}{E I} \quad \text { Wait! Not entirely correct! }
$$

## Example 2

## Previous angle relative to fixed support B



$$
\begin{aligned}
\theta_{F}^{c w+} & =\frac{q L^{3}}{48 E I}-\frac{M_{F} L}{E I}+\theta_{B} \\
\theta_{F}^{c w+} & =\frac{7 q L^{3}}{48 E I}-\frac{2 M_{F} L}{E I}+\frac{H_{F} L^{2}}{2 E I}=0
\end{aligned}
$$

## Example 2

Recall equilibrium:

Solve:

$$
\begin{aligned}
& \sum \overrightarrow{F+} \Rightarrow H_{A}=-H_{F} \\
& \sum F^{\uparrow+} \Rightarrow V_{A}=\frac{q L}{2}
\end{aligned}
$$

Recall compatibility:
$\theta_{F}=0, \quad \mathrm{v}_{B}=0$

$$
\sum M_{A}^{c c w+} \Rightarrow M_{A}+\frac{q L^{2}}{8}+H_{F} L=M_{F}
$$

$$
\begin{aligned}
& \theta_{F}^{c w+}=\frac{7 q L^{3}}{48 E I}-\frac{2 M_{F} L}{E I}+\frac{H_{F} L^{2}}{2 E I}=0 \\
& \vec{v}_{B}^{+}=\frac{H_{F} L^{3}}{3 E I}-\frac{M_{F} L^{2}}{2 E I}+\frac{q L^{4}}{16 E I}=0
\end{aligned}
$$

$$
\begin{array}{ll}
H_{F}=-\frac{q L}{8} & H_{A}=\frac{q L}{8} \\
M_{F}=\frac{q L^{2}}{24} & M_{A}=-\frac{2 q L^{2}}{3}
\end{array}
$$

## Example 2



$$
\begin{aligned}
& H_{F}=-\frac{q L}{8} \\
& M_{F}=\frac{q L^{2}}{24} \\
& H_{A}=\frac{q L}{8} \\
& V_{A}=\frac{q L}{2} \\
& M_{A}=-\frac{2 q L^{2}}{3}
\end{aligned}
$$

## For next time

|  | First Lecture |  | Second Lecture |  | coz |  | Instruction |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Week | Topics | Preparation | Topics | Preparation | Assignment | Due Date | Activity |
| 3.1 | Introduction | None | Stress, Strain, Hooke's Law | L.U. 1 (all) | none | none | no Instruction |
| 3.2 | Axial loading and static indetermenacy | L.U. 2 (all) | Torsion of circular shafts | L.U. 3.1-3.2 | coz1 | 18/02/2016 | Mock exam 1 |
| 3.3 | Torsion of thin-walled shafts | L.U. 3.3 | Bending stresses in beams | L.U. 4 (all) | COZ2 | 25/02/2016 | Peer grading 1 |
| 3.4 | Transverse shear stresces in heams | L.U. 5.1 | Shear stresses in thinwalled heams | L.U. 5.2 | coz3 | 03/03/2016 | Mock exam 2 |
| 3.5 | Combined loading | L.U. 6 (all) | Stress transformations \& Failure criteria | L.U. 7 (all) | coz4 | 10/03/2016 | Peer Grading 2 |
| 3.6 | Eeam deflections by integration | L.U. 8.1 | Discontinuity functions and | L.U. 8.2-8.3 | COZ5 | 17/03/2016 | Mock exam 3 |
| 3.7 | statıcally <br> indeterminate beams | L.U. $\mathrm{B}$. | keview | None | COZ6 | 24/03/2016 | Peer grading 3 |
| 3.8 | Study for Exam |  |  |  |  |  |  |
| 3.9 | ExamII - Fidday April 8 th@ 13:30 |  |  |  |  |  |  |

L.U. = Learning unit 'refer to blackboard site for learning units)

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