

# oe4625 Dredge Pumps and Slurry Transport



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# 4. MODELING OF STRATIFIED MIXTURE FLOWS (Heterogeneous Flows)

**EMPIRICAL MODELING**

**THEORETICAL MODELING**

# EMPIRICAL MODELING

**DURAND  
FÜHRBÖTER  
JUFIN-LOPATIN  
WILSON-GIW  
MTI**

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# Empirical Model by Durand et al

## A. Experimental observations

- - a wide range of slurry flow conditions including several pipeline sizes and sorts of sand and gravel
- - low concentrated slurries ( $C_{vd}$  up to 22%)

## B. Construction of the Durand correlation for $I_m$ :

- - the solids effect  $I_m - I_f$  decreases gradually with increasing mean slurry velocity  $V_m$  in flow of constant delivered volumetric concentration of solids  $C_{vd}$
- - the solids effect  $I_m - I_f$  increases approximately linearly with increasing  $C_{vd}$  at constant  $V_m$ .

# Empirical Model by Durand et al

## B. Construction of the correlation for $I_m$ (cont'):

- The latter condition is written as  $I_m - I_f = \text{const.} C_{vd}$  and generalized in the dimensionless group  $\Phi$ ,

$$\Phi = \frac{I_m - I_f}{I_f C_{vd}} = \text{const.}$$

The flow coefficient  $\Phi$  is not constant for slurry flows of different pipeline size  $D$ , solids size  $d$ , or flow velocity  $V_m$ .

# Empirical Model by Durand et al

## B. Construction of the correlation for $I_m$ (cont'):

The flow coefficient  $\Phi$  varies with  $D$ ,  $d$ , and  $V_m$ .  
An effect of these parameters is introduced to the correlation using the dimensionless groups

- the Froude number for mixture flow

$$Fr^2 = \frac{V_m^2}{gD}$$

and

- the Froude number for a solid particle

$$Fr_{vt}^2 = \frac{v_t^2}{gd}$$

# Intermezzo: Froude Number

The dimensionless group **Fr, Froude number**, is a ratio of the inertial forces and the gravitational forces acting on a certain control volume (this is here a pipeline flow)

$$Fr^2 = \frac{V^2}{gD} = \frac{\textit{inertial.force}}{\textit{gravitational.force}}$$

The *Froude number* is a criterion of dynamic similarity in different flow conditions for flows with a dominant effect of inertia and gravity.

# Empirical Model by Durand et al

The new dimensionless group  $\Psi$  (covering the effect of  $D, d, V_m$ ):

$$\Psi = Fr^2 Fr_{vt}^{-1} = \frac{V_m^2 \sqrt{gd}}{gD v_t}$$

A general empirical relationship was established by Durand et al for pressure drop due to friction in pipeline flow of slurry

$$\Phi = K\Psi^n$$

where  $K$  and  $n$  are *empirical coefficients*. Their values have to be found by experiments.



# Empirical Model by Durand et al

The  $\Phi - \Psi$  relationship is determined using

- the hyperbolic curve in the  $\Phi$ - $\Psi$  plot by Durand & Condolios (see next slide) or
- the curve approximation giving  $K = 180$  and  $n = -1.5$ .

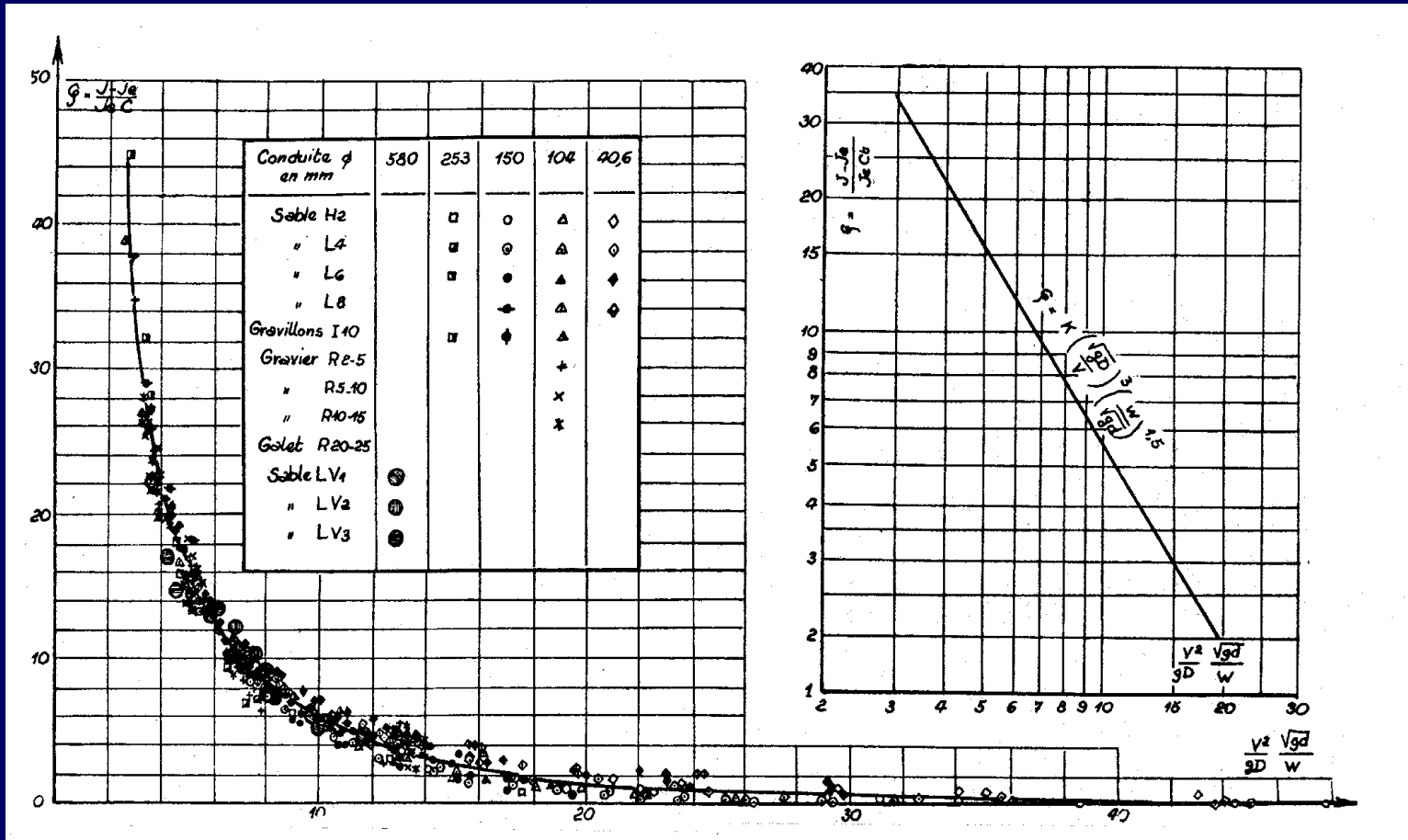
The ultimate correlation obtained by Durand et al:

$$\frac{I_m - I_f}{I_f C_{vd}} = 180 \left( \frac{V_m^2 \sqrt{gd}}{gD v_t} \right)^{-1.5}$$

The equation is recommended for the region  $4 < \Psi < 15$  (medium and medium to coarse sand).

# Empirical Model by Durand & Condolios

Dimensionless group  $\Phi$  [-]



Dimensionless group  $\Psi$  [-]

# Empirical Model by Durand et al

## A. Experimental observations for $V_{dl}$ :

- visual observations of the initial formation of a stationary bed in pipelines for different mixture flow conditions.

## B. Construction of the Durand correlation for $V_{dl}$ :

- the Froude number for flow above the stationary bed remained constant when *a stationary bed was formed and gradually became thicker under decreasing  $V_m$  in a pipeline.*

# Empirical Model by Durand et al

## B. Construction of the $V_{dl}$ correlation (cont'):

- The Froude number is based on the velocity above a stationary bed,  $V_e$ , and on the hydraulic radius,  $R_h$ , of discharging area above the stationary bed.

$$Fr^2 = \frac{V_e^2}{gR_h}$$

- For flow conditions at the beginning of the stationary bed ( $V_e = V_m = V_{dl}$  and  $D = 4R_h$ ) this condition is written as

$$Fr^2 = \frac{V_{dl}^2}{gD} = \text{const.}$$

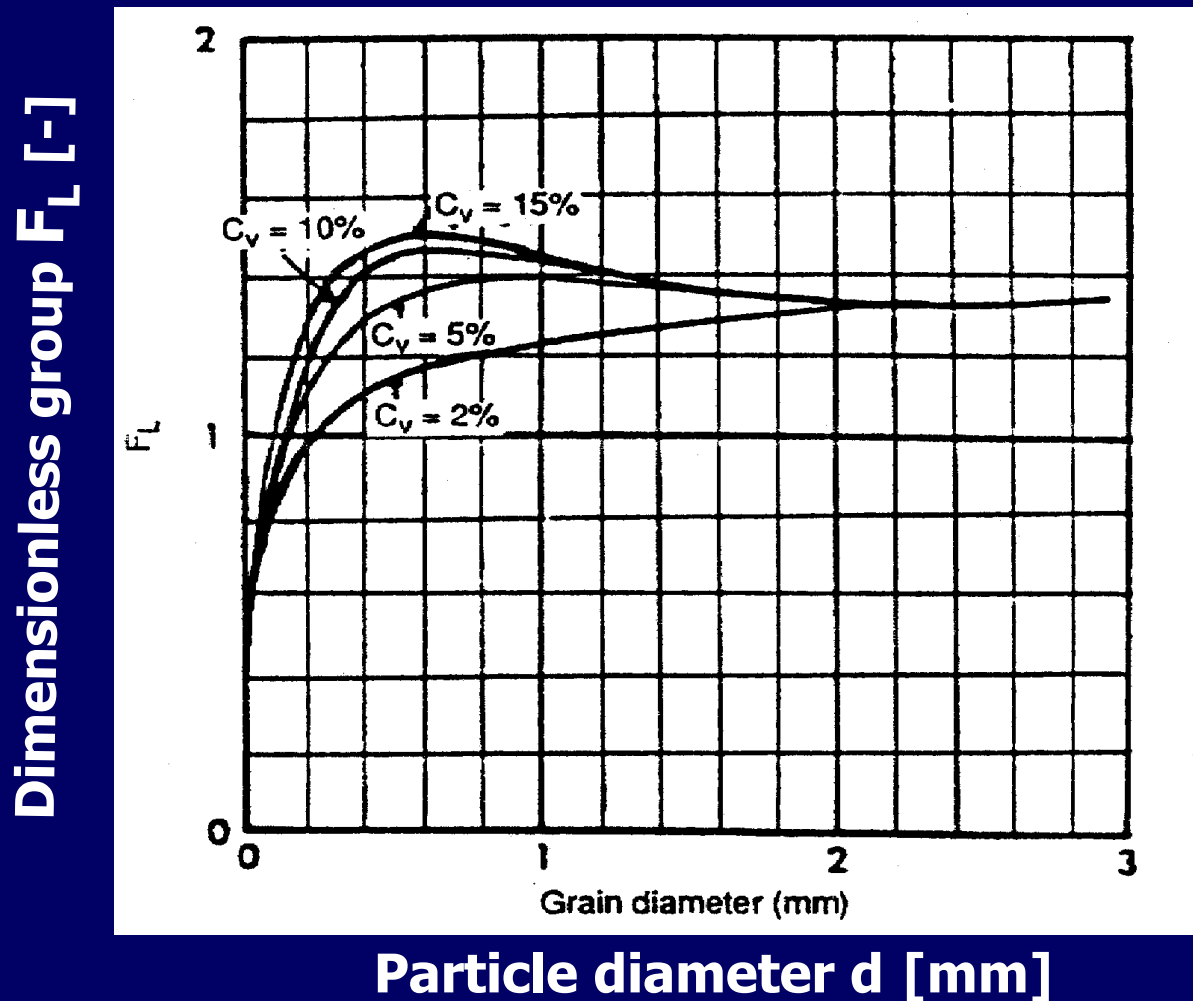
# Empirical Model by Durand et al

## B. Construction of the $V_{dl}$ correlation (cont'):

- An effect of various particle diameters  $D$  and delivered concentrations  $C_{vd}$  on the value of the  $V_{dl}$  was expressed in the empirical relationship  $F_L = f(d, C_{vd})$  presented as a graph (see next slide).
- The correlation for the deposition-limit velocity by Durand et al is written as

$$V_{dl} = F_L \sqrt{2g(S_s - 1)D}$$

# Empirical Model by Durand et al



# Empirical Model by Führböter

## A. Experimental observations

- slurry flow conditions in a 300 mm laboratory pipeline for sand and gravel of particle size range between 0.15 mm and 1.8 mm.

## B. Construction of the Führböter correlation for $I_m$ :

- The correlation was found

$$I_m - I_f = S_k \frac{C_{vi}}{V_m}$$

in which  $S_k$  was *the empirical coefficient* dependent on solids properties.

# Empirical Model by Führböter

## B. Construction of the correlation for $I_m$ (cont'):

- Practical calculations are done for  $C_{vd}$  instead of  $C_{vi}$ , thus the slip effect is incorporated to obtain  $C_{vd}$  in the model equation. The constant value of the slip ratio  $C_{vd}/C_{vi} = 0.65$  is considered to hold for all mixture flow conditions. The transport factor  $S_{kt}$  is obtained by  $S_{kt} = S_k C_{vi}/C_{vd}$ .

- The Führböter correlation is 
$$I_m - I_f = S_{kt} \frac{C_{vd}}{V_m}$$

Values of the factor  $S_{kt}$  are determined empirically (by experiment).



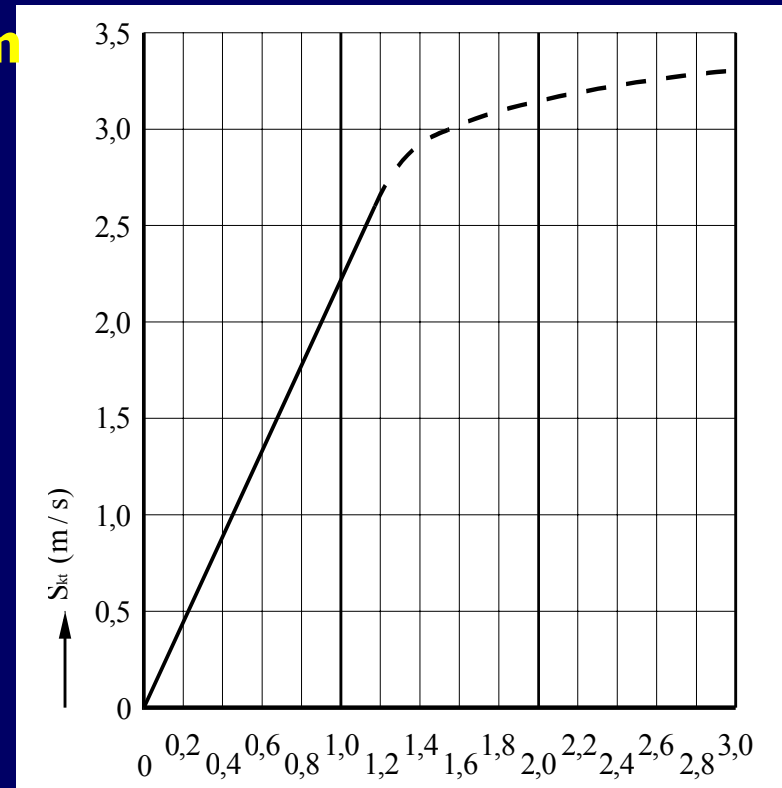
# Empirical Model by Führböter

$S_{kt} = 2.59 d_m - 0.037$  for  $0.2 < d_m < 1.1$  mm

$S_{kt}$  from graph for  $1.1 < d_m < 3.0$  mm

$S_{kt}$  is approximately 3.3 for  $d_m > 3.0$  mm.

Coefficient  $S_{kt}$  [m/s]



Particle diameter  $d$  [mm]

# Empirical Model by Jufin and Lopatin

## A. Experimental observations

- broad data base including data from dredging installations.

## B. Construction of the Jufin correlation for $I_m$ :

- The assumption based on experiments sounds: the hydraulic gradient  $I_m$  at the minimum velocity  $V_{min}$  equals  $I_m = 3I_f$ .

- The general correlation is 
$$I_m = I_f \left( 1 + 2 \left[ \frac{V_{min}}{V_m} \right]^3 \right)$$

# Empirical Model by Jufin and Lopatin

## C. The minimum velocity by Jufin:

The minimum velocity  $V_{min}$  is determined using the empirical correlation

$$V_{min} = 5.3 \left( C_{vd} \cdot \psi^* \cdot D \right)^{\frac{1}{6}}$$

in which the particle settling parameter  $\psi^* = \mathbf{f}(d)$  is either from Table or calculated as the modified Froude number of a solid particle:

$$\psi^* = Fr_{vt}^{1.5} = \left( \frac{v_t}{\sqrt{gd}} \right)^{1.5}$$

# Empirical Model by Jufin and Lopatin

## D. The deposition-limit velocity by Jufin:

The deposition-limit velocity  $V_{dl}$  is given by the empirical correlation

$$V_{dl} = 8.3D^{\frac{1}{3}} \left( C_{vd} \cdot \psi^* \right)^{\frac{1}{6}}$$

in which the particle settling parameter  $\psi^* = \mathbf{f}(\mathbf{d})$  is either from Table or calculated as modified Froude number of a solid particle:

$$\psi^* = Fr_{vt}^{1.5} = \left( \frac{v_t}{\sqrt{gd}} \right)^{1.5}$$

# Empirical Model by Wilson & GIW

## A. Experimental observations

- data from the GIW laboratory circuits (diameters 200 mm and 440 mm, for medium to coarse sands in mixtures of delivered concentrations up to 0.16.

## B. Construction of the correlation for $I_m$ :

The model considers the *heterogeneous flow* as a transition between two extreme flows governed by different mechanisms for support of a solid particle in the carrying-liquid flow:

- the *fully-stratified flow* (all particles in a contact load) and
- the *fully-suspended flow* (all particles in a suspended load).

# Empirical Model by Wilson & GIW

## B. Construction of the correlation for $I_m$ (cont'):

The theoretical consideration:

- The energy dissipation due to the presence of solid particles in a carrier flow is predominantly due to *mechanical friction* between contact-load particles and a pipeline wall.
- A resisting force of the contact bed against the carrier flow is related to the *submerged weight of the bed* via the coefficient of mechanical friction.

# Empirical Model by Wilson & GIW

## B. Construction of the correlation for $I_m$ (cont'):

- Thus at  $V_m = V_{50}$  (the mean slurry velocity at which one half of the transported solid particles contribute to a suspended load and one half to a contact load velocity) the solids effect is due to the submerged weight of the moving bed containing one half of the total solid fraction multiplied by the friction coefficient,  $\mu_s$ ,

$$I_m - I_f = 0.5 \mu_s C_{vd}(S_s - 1).$$

# Empirical Model by Wilson & GIW

## B. Construction of the correlation for $I_m$ (cont'):

The experimental experience:

- A linear relationship between measured values of the ratio

$$\frac{I_m - I_f}{C_{vd} (S_s - 1)}$$

and the mean mixture velocity  $V_m$  in plotted within the log-log co-ordinates.

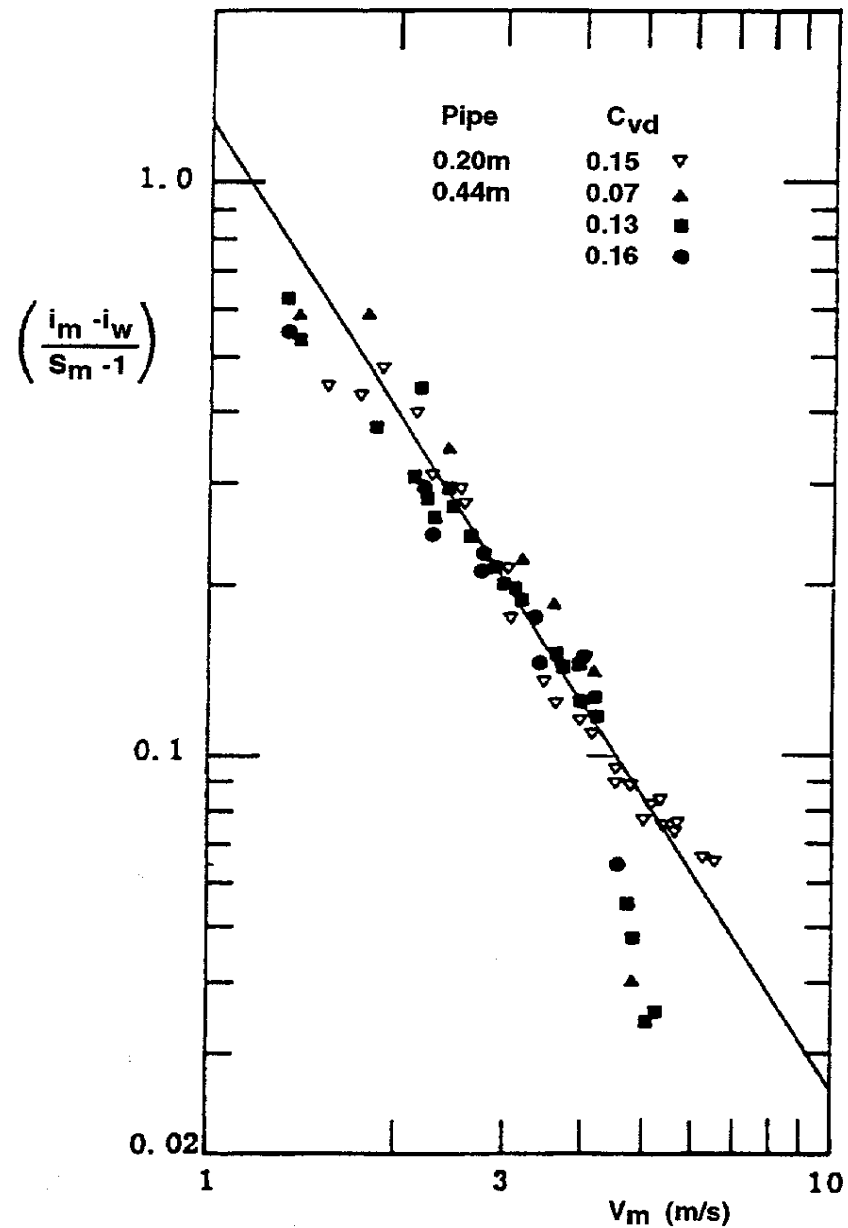
The relationship was found the same for flows of different concentrations in pipes of different sizes.



# Wilson & GIW

$$\frac{I_m - I_f}{C_{vd} (S_s - 1)} \text{ versus } V_m$$

Masonry sand mixture  
( $d_{50} = 0.42$  mm),  
after Clift et al. (1982).



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# Empirical Model by Wilson & GIW

## B. Construction of the correlation for $I_m$ (cont'):

Generalization:

The correlation for various mean mixture velocities:

$$\frac{I_m - I_f}{C_{vd} (S_s - 1)} = 0.5 \mu_s \left( \frac{V_m}{V_{50}} \right)^{-M} = 0.22 \left( \frac{V_m}{V_{50}} \right)^{-M}$$

# Empirical Model by Wilson & GIW

## B. Construction of the correlation for $I_m$ (cont'):

Generalization (cont'):

$V_{50}$  should be obtained experimentally or estimated roughly by the approximation:

$$V_{50} \approx 3.93(d_{50})^{0.35} \left( \frac{S_s - 1}{1.65} \right)^{0.45}$$

in which  $d_{50}$  [mm] and  $V_{50}$  [m/s].

The exponent  $M$  is given by:

( $M$  should not exceed 1.7, the value for narrow-graded solids, nor fall below 0.25).

$$M \approx \left[ \ln \left( \frac{d_{85}}{d_{50}} \right) \right]^{-1}$$

# Empirical Model for Critical Velocity (MTI)

## A. Definition of critical velocity according to MTI Holland

The *critical velocity* is the threshold velocity between the "fully suspended heterogeneous flow" regime and the regime of "flow with the first particles settling to the bottom" of a pipeline.

This velocity is suggested to be the lowest acceptable velocity for a economic and safe operation of a dredging pipeline.

$$V_{cr} \sim V_{min}$$

# Empirical Model for Critical Velocity (MTI)

## B. Correlation for $V_{cr}$ :

$$V_{crit} = 1.7 \left( 5 - \frac{1}{\sqrt{d_{mf}}} \right) \sqrt{D} \left( \frac{C_{vd}}{C_{vd} + 0.1} \right)^{\frac{1}{6}} \sqrt{\frac{S_s - 1}{1.65}}$$

In the equation  $d_{mf}$  [mm],  $D$  [m] and  $V_{cr}$  [m/s].

# Empirical Model for Critical Velocity (MTI)

## C. Diagram for $V_{cr}$ :

